UNIVERSIDADE FEDERAL DE MINAS GERAIS ESCOLA DE ENGENHARIA DEPARTAMENTO DE ENGENHARIA DE ESTRUTURAS CURSO DE ESPECIALIZAÇÃO EM ESTRUTURAS

LAGE COGUMELO – DIMENSIONAMENTO Á FLEXÃO E PUNÇÃO

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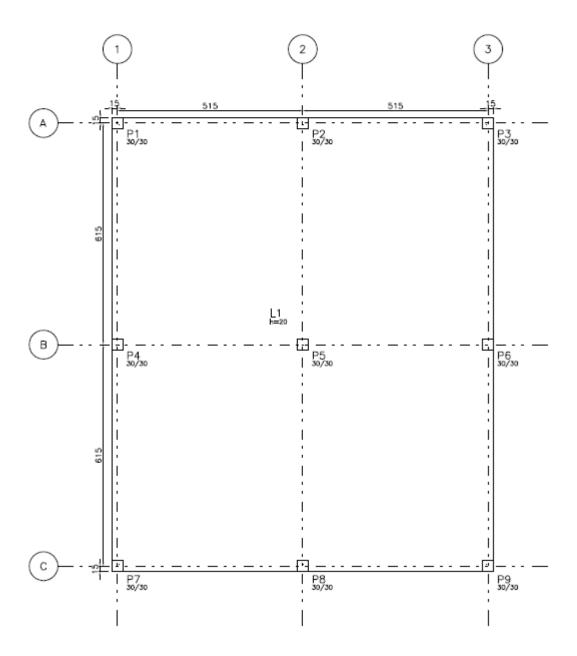
Ser engenheiro civil é motivo de grande satisfação e de muita responsabilidade. A qualidade de um serviço de engenharia está diretamente relacionada com a capacidade técnica e prática dos profissionais envolvidos. Cursar a Especialização em Estruturas tem como propósito aumentar o embasamento teórico no referido assunto, crescer a rede de contatos com colegas de profissão e com os mestres da UFMG que ensinam com tanta sabedoria e diligência.

Após ser graduado pela FUNEDI / UEMG no fim de 2012, ingressei como aluno regular no curso de Especialização em Estruturas. As disciplinas cursadas foram: Projeto de Estruturas de Concreto I, Projeto de Estruturas de Aço I, Projeto de Estruturas de Aço II, Projeto de Estruturas de Aço com Perfis Estruturais Formados a Frio, Projeto de Estruturas Mistas de Aço e Concreto, Projeto de Estruturas de Fundação e Concepção de Estruturas de Edifícios.

Para conclusão do Curso de Especialização, além da obrigatoriedade de cursar oito disciplinas afins, é necessária a entrega de um Trabalho Prático Final. A proposta desenvolvida neste trabalho foi dimensionar e detalhar, à flexão e à punção, uma laje lisa de concreto armado. Esse tema foi escolhido visando aperfeiçoamento no procedimento de cálculo desse tipo de laje. Todo processo de cálculo utilizado está de acordo com a NBR 6118:2003. As dimensões da laje são de 10,60m x 12,60m com 9 pilares de apoio. A primeira etapa do Trabalho prático é dimensionar e detalhar a laje lisa à flexão. A segunda etapa é verificar a punção todas as regiões da laje que estão em contado direto com os pilares.

O curso de Especialização em Estruturas da UFMG superou as minhas expectativas, me dando mais segurança e aumentando a qualidade dos meus serviços de engenharia, o que me fez um engenheiro mais satisfeito com a profissão. Agradeço a todos os mestres do ensino e a todos os funcionários desta instituição. Em especial, agradeço ao grande amigo Ney Amorim Silva, meu professor e orientador neste trabalho final, por toda a amizade construída, por toda a dedicação e disponibilidade dedicada a mim.

Fôrma da laje lisa



1 – Espessura da laje:

$$Lx = 515cm$$

$$Ly = 615cm$$

$$L \geq \begin{cases} Lx = 515cm \\ Ly = 615cm \end{cases} \therefore L = 615cm$$

$$\frac{L}{36} \leq h \leq \frac{L}{40} \div \frac{615cm}{36} \leq h \leq \frac{615cm}{40} \div 15cm \leq h \leq 17cm$$

$$h = 16cm : h_{min} = 16cm : ok!$$

2 – Dimensões dos pilares:

Obs: A dimensão "a" do pilar é paralela ao lado "Lx" da laje e a dimensão "b" do pilar é paralela ao lado "Ly" da laje.

$$a \ge \begin{cases} \frac{Lx}{20} = \frac{515cm}{20} = 25,75cm \\ \frac{h_{pilar}}{15} = \frac{(296cm - 16cm)}{15} = 18,67cm \end{cases} \therefore a = 30cm$$

$$b \ge \begin{cases} \frac{Ly}{20} = \frac{615cm}{20} = 30,75cm \cong 30cm \\ \frac{h_{pilar}}{15} = \frac{(296cm - 16cm)}{15} = 18,67cm \end{cases} \therefore b = 30cm$$

3 – Carregamentos:

3.1 – Alvenaria nas bordas:

$$0,20m \times 2,80m \times 13 \frac{KN}{m^3} = 7,28 \frac{KN}{m}$$

3.2 – Cargas distribuídas sobre a área da laje:

3.2.1 – Peso próprio da laje:

$$2500 \frac{kgf}{m^3} \times 0,16m = 400 \frac{Kgf}{m^2}$$

3.2.2 – Revestimento:

$$100 \; \frac{kgf}{m^2}$$

3.2.3 – Sobrecarga:

$$300 \frac{kgf}{m^2}$$

3.2.4 – Carga característica total:

$$400\frac{kgf}{m^2} + 100\frac{kgf}{m^2} + 300\frac{kgf}{m^2} = 800\frac{kgf}{m^2} = 8,0\frac{KN}{m^2}$$

4 – Carregamento nos pórticos:

Obs: Considera-se "q" como carga acidental e "g" como carga permanente.

$$q = 300 \frac{kgf}{m^2}$$

$$g = 400 \frac{kgf}{m^2} + 100 \frac{kgf}{m^2} = 500 \frac{kgf}{m^2}$$

$$q = 300 \frac{kgf}{m^2} \leq 0.75 \times g = 0.75 \times 500 \frac{kgf}{m^2} = 375 \frac{kgf}{m^2}$$

Portanto, $q \le 0.75g$. Esta situação define qual o modelo de carregamento a ser adotado.

4.1 – Direção x:

4.1.1 – Inércia das "vigas" dos eixos A e C:

$$Iv = \frac{3,225m \times (0,16m)^3}{12} = 1,10 \times 10^{-3}m^4$$

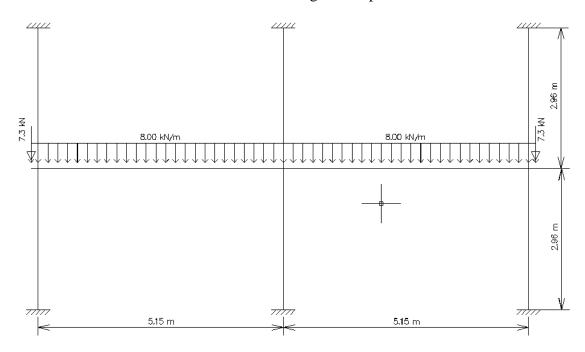
4.1.2 – Inércia das "vigas" do eixo B:

$$Iv = \frac{6,15m \times (0,16m)^3}{12} = 2,10 \times 10^{-3}m^4$$

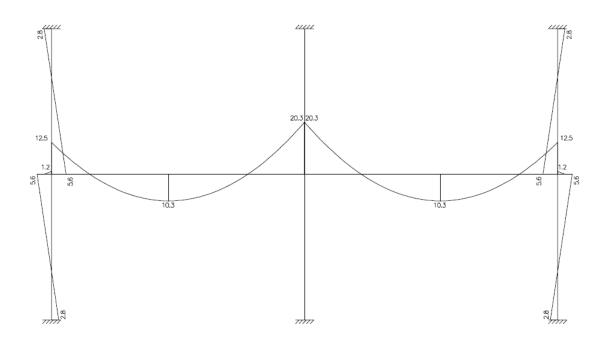
4.1.3 – Inércia dos "pilares":

$$Ip = \frac{0,30m \times (0,30m)^3}{12} = 6,75 \times 10^{-4}m^4$$

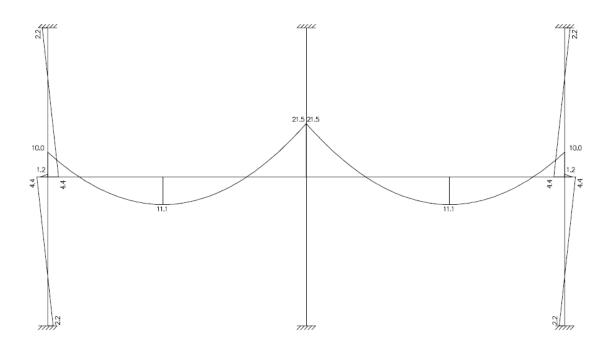
4.1.4 – Modelo estrutural considerando o carregamento por faixa de 1m:



4.1.5 – Diagrama de momento fletor referente aos Eixos A e C (KN.m):



4.1.6 – Diagrama de momento fletor referente ao Eixo B (KN.m):



- 4.2 Direção y:
- 4.2.1 Inércia das "vigas" dos eixos 1 e 3:

$$Iv = \frac{2,725m \times (0,16m)^3}{12} = 9,30 \times 10^{-4}m^4$$

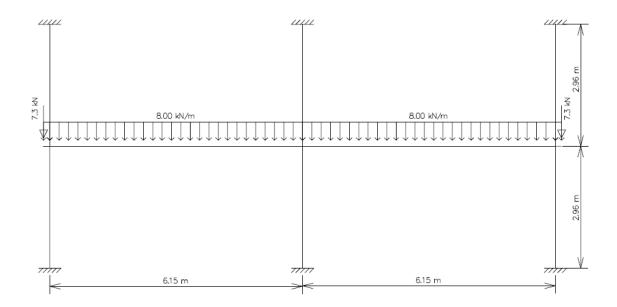
4.2.2 – Inércia das "vigas" do eixo 2:

$$Iv = \frac{5,15m \times (0,16m)^3}{12} = 1,76 \times 10^{-3}m^4$$

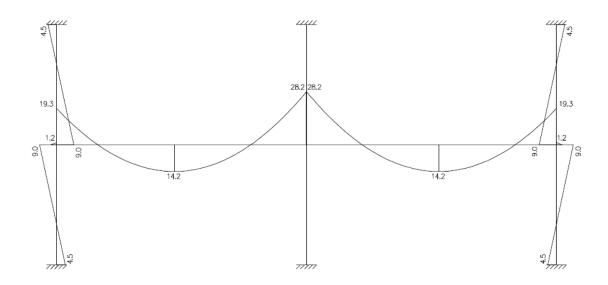
4.2.3 – Inércia dos "pilares":

$$Ip = \frac{0,30m \times (0,30m)^3}{12} = 6,75 \times 10^{-4}m^4$$

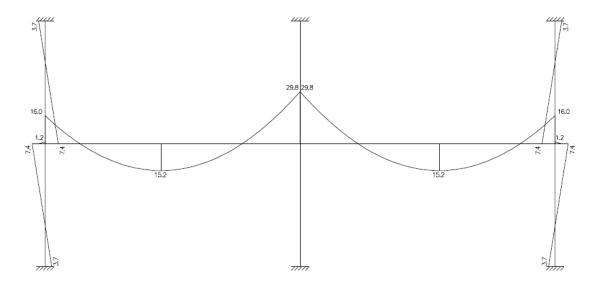
4.2.4 – Modelo estrutural considerando o carregamento por faixa de 1m:



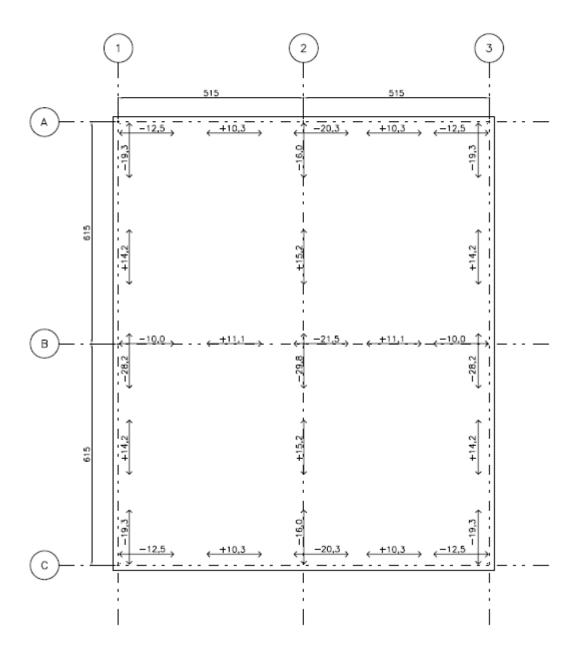
4.2.5 – Diagrama de momento fletor referente aos Eixos 1 e 3 (KN.m):



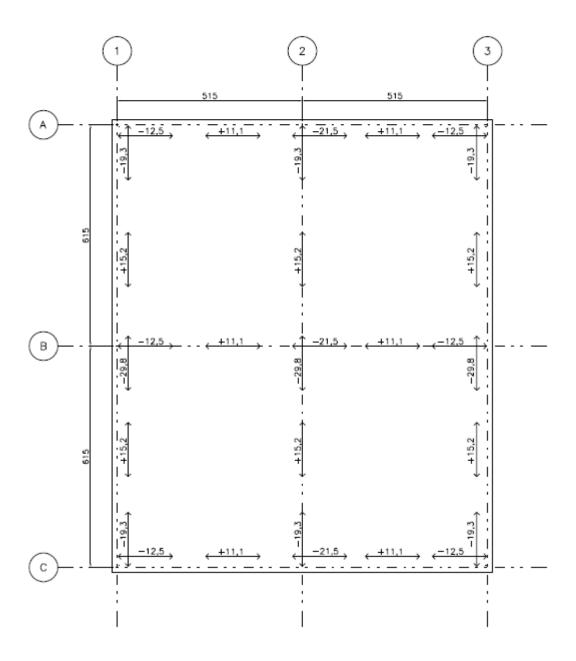
4.2.6 – Diagrama de momento fletor referente ao Eixo 2 (KN.m):



Momentos calculados (KN.m)



Momentos adotados (KN.m)



5 – Dimensionamento:

5.1 – Largura das faixas na direção x:

$$\frac{615cm}{4} = 153,75cm$$

5.2 – Largura das faixas na direção y:

$$\frac{515cm}{4} = 128,75cm$$

5.3 – Altura útil da laje:

$$H = 16cm : d' = 4,5cm$$

$$d = H - d' = 16cm - 4,5cm = 11,5cm$$

5.4 – Área de aço mínima:

$$As_{min} = 0.15\%.\,b.\,h = 0.15\%\times100cm\times16cm = 2.4\,\frac{cm^2}{m\;de\;laje}$$

5.5 – Área de aço na direção x:

5.5.1 – Faixas externas:

MOMENTO NO PAINEL (kN.m)	% POR FAIXA	MOMENTOS POR FAIXA (kN.m)	d (cm)	fc (KN/cm²)	b (cm)	k	As total (cm²)	As por metro (cm²)	DIÂMETRO
-12,5 x 6,15 = -76,9	37,5%	-29,5	11,5	1,52	153,75	0,13	8,64	5,62	Ø10 c/ 14cm
-21,5 x 6,15 = -132,2	37,5%	-49,6	11,5	1,52	153,75	0,22	15,56	10,12	Ø12,5 c/ 12cm
11,1 x 6,15 = 68,3	27,5%	18,8	11,5	1,52	153,75	0,09	5,84	3,8	Ø8 c/ 13cm

5.5.2 Faixas internas:

MOMENTO NO PAINEL (kN.m)	% POR FAIXA	MOMENTOS POR FAIXA (kN.m)	d (cm)	fc (KN/cm ²)	b (cm)	k	As total (cm²)	As por metro (cm²)	DIÂMETRO
-12,5 x 6,15 = -76,9	12,5%	-9,6	11,5	1,52	153,75	0,04	2,52	*2,40	Ø8 c/ 20cm
-21,5 x 6,15 = -132,2	12,5%	-16,5	11,5	1,52	153,75	0,07	4,49	2,92	Ø8 c/ 17cm
11,1 x 6,15 = 68,3	22,5%	15,4	11,5	1,52	153,75	0,07	4,49	2,92	Ø8 c/ 17cm

^{*} Área de aço mínima.

5.6 – Área de aço na direção y:

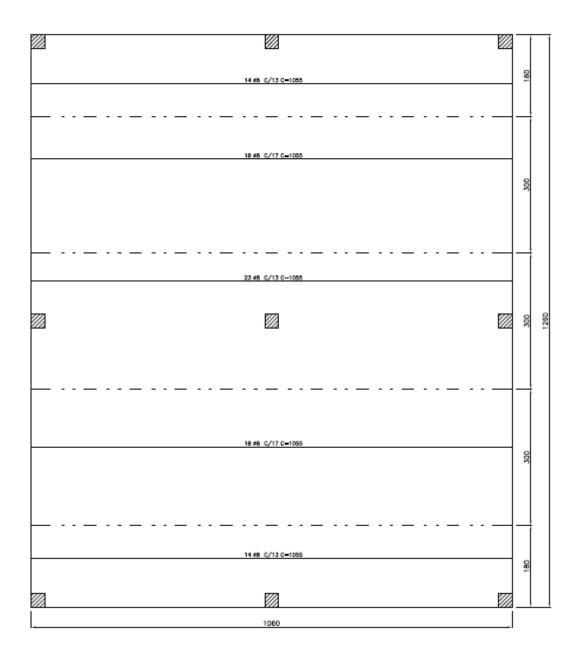
5.6.1 – Faixas externas:

MOMENTO NO PAINEL (kN.m)	% POR FAIXA	MOMENTOS POR FAIXA (kN.m)	d (cm)	fc (KN/cm²)	b (cm)	k	As total (cm²)	As por metro (cm²)	DIÂMETRO
-19,3 x 5,15 = -99,4	37,5%	37,3	11,5	1,52	128,75	0,2	11,67	9,06	Ø12,5 c/ 13cm
-29,8 x 5,15 = -153,5	37,5%	57,6	11,5	1,52	128,75	0,31	19,85	15,42	Ø16 c/ 13cm
15,2 x 5,15 = 78,3	27,5%	21,5	11,5	1,52	128,75	0,12	6,64	5,16	Ø10 c/ 15cm

5.6.2 – Faixas internas:

MOMENTO NO PAINEL (kN.m)	% POR FAIXA	MOMENTOS POR FAIXA (kN.m)	d (cm)	fc (KN/cm²)	b (cm)	k	As total (cm²)	As por metro (cm²)	DIÂMETRO
-19,3 x 5,15 = -99,4	12,5%	12,4	11,5	1,52	128,75	0,07	3,76	2,92	Ø8 c/ 17cm
-29,8 x 5,15 = -153,5	12,5%	19,2	11,5	1,52	128,75	0,10	5,46	4,24	Ø10 c/ 18cm
15,2 x 5,15 = 78,3	22,5%	17,6	11,5	1,52	128,75	0,10	5,46	4,24	Ø10 c/ 18cm

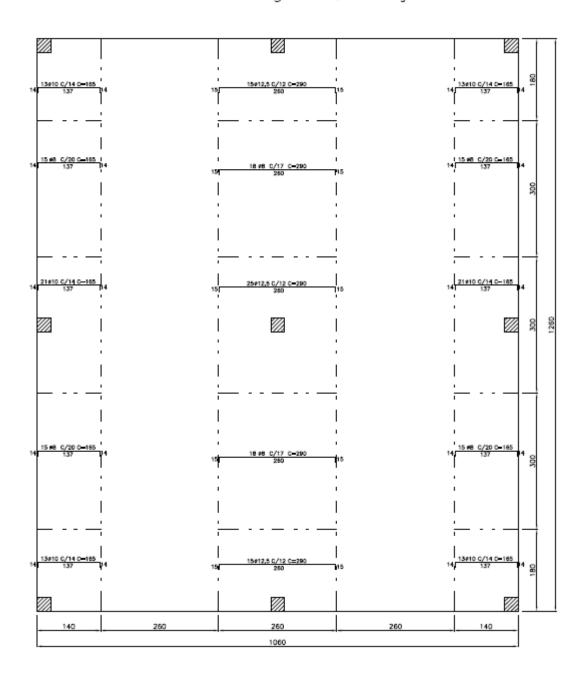
Armadura positiva, direção x



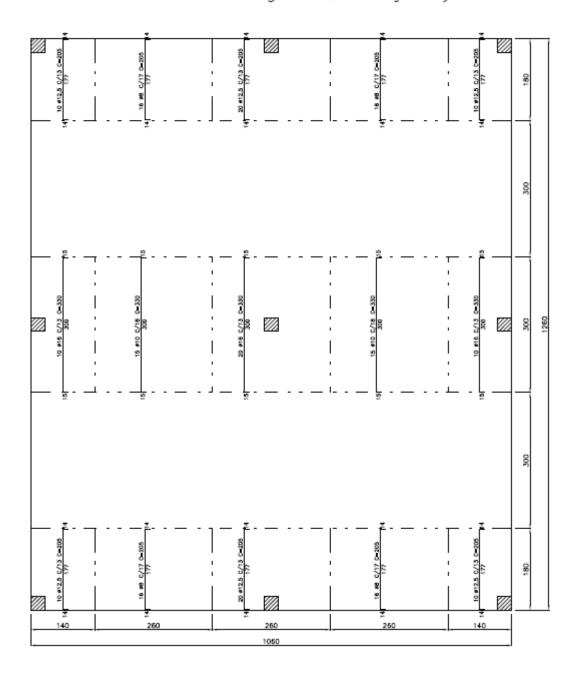
Armadura positiva, direção y

0021-0 C/20 C+1250	15 410 G/18 C=1256	10 sto c/15 c-1255	15 #10 C/18 C=1206	10 e10 C/15 C=1255					
. 140	140 260 260 140								
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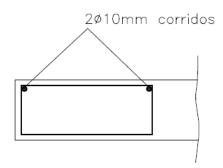
Armadura negativa, direção x

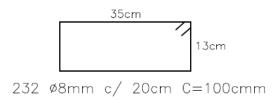


Armadura negativa, direção y



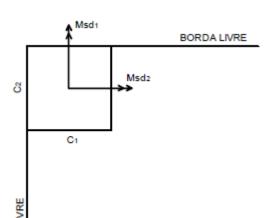
5.7 – Reforços de borda:





PUNÇÃO

Verificação dos pilares P1, P3, P7 e P9, usando como referência o pilar P1:



1 - Dados:

$$C_1 = 40cm$$

$$C_2 = 40cm$$

$$d = h - d' = 16cm - 4,5cm = 11,5cm$$

$$Fsd = Fz. 1,4 = 113,63KN \times 1,4 = 159,08KN (Compressão)$$

$$Msd_1 = M_v. 1,4 = 1806KN.cm \times 1,4 = 2528,4KN.cm$$

$$Msd_2 = M_x$$
. 1,4 = 2453KN. cm × 1,4 = 3434,2KN. cm

$$a_{01} \leq \begin{cases} 1,5.\,d = 1,5 \times 11,5cm = 17,25cm \\ C_1 = 40cm \end{cases} :: a_{01} = 17,25cm$$

$$\alpha_{02} \leq \begin{cases} 1.5.\,d = 1.5 \times 11.5cm = 17.25cm \\ C_2 = 40cm \end{cases} :: \alpha_{02} = 17.25cm$$

$$a_1 \leq \begin{cases} 1.5.\,d = 1.5 \times 11.5cm = 17.25cm \\ 0.5.\,C_1 = 0.5 \times 40cm = 20cm \end{cases} \therefore \, a_1 = 17.25cm$$

$$a_2 \leq \begin{cases} 1.5.\,d = 1.5 \times 11.5cm = 17.25cm \\ 0.5.\,C_2 = 0.5 \times 40cm = 20cm \end{cases} \therefore \, a_2 = 17.25cm$$

2 – Verificação do Contorno C:

$$u^* = a_{01} + a_{02} = 17,25cm + 17,25cm = 34,5cm$$

2.1 – Análise de Msd₁:

$$wp_1 = \frac{{C_1}^2}{4} + \frac{{C_1}.{C_2}}{2} = \frac{(40cm)^2}{4} + \frac{40cm \times 40cm}{2} = 1200cm^2$$

$$e_1^* = \frac{C_1 \cdot a_1 - a_1^2 + a_2 \cdot C_1}{2 \cdot (a_1 + a_2)} = \frac{40cm \times 17,25cm - (17,25cm)^2 + 17,25cm \times 40cm}{2 \times (17,25cm + 17,25cm)}$$

$$e_1^* = 15,69cm$$

$$Msd_1^* = Fsd.e_1^* = 159,08KN \times 15,69cm = 2495,97KN.cm$$

$$Msd_{1_{fim}} \geq \begin{cases} 0 \\ Msd_{1} - Msd_{1}^{*} = 2528,4KN.\,cm - 2495,97KN.\,cm = 32,43KN.\,cm \end{cases}$$

$$Msd_{1_{fim}} = 32,43KN.cm$$

$$\frac{C_1}{C_2} = \frac{40cm}{40cm} = 1 :: K_1 = 0,60$$

$$Tsd_1 = \frac{Fsd}{u^*.d} + K_1.\frac{Msd_{1_{fim}}}{wp_1.d} = \frac{159,08KN}{34,5cm \times 11,5cm} + 0.6 \times \frac{32,43KN.cm}{1200cm^2 \times 11,5cm}$$

$$Tsd_1 = 0.402 \frac{KN}{cm^2} = 4.02MPa$$

$$Trd_2 = 0.27. \\ \propto_v. \\ fcd = 0.27 \\ \times \left(1 - \frac{fck}{250}\right) \\ \times \left(\frac{fck}{1.4}\right) = 0.27 \\ \times \left(1 - \frac{25MPa}{250}\right) \\ \times \left(\frac{25MPa}{1.4}\right) \\ = 0.27 \\ \times \left(1 - \frac{25MPa}{250}\right) \\ \times \left(\frac{25MPa}{1.4}\right) \\ \times \left(\frac{1}{1.4}\right) \\ \times \left(\frac{1}{1.4}\right)$$

$$Trd_2 = 4,34MPa$$

$$Tsd_1 = 4.02MPa < Trd_2 = 4.34MPa : ok!$$

2.2 – Análise de Msd₂:

$$wp_2 = \frac{{C_2}^2}{4} + \frac{{C_2} \cdot {C_1}}{2} = \frac{(40cm)^2}{4} + \frac{40cm \times 40cm}{2} = 1200cm^2$$

$$e_2^* = \frac{C_2 \cdot a_2 - a_2^2 + a_1 \cdot C_2}{2 \cdot (a_2 + a_1)} = \frac{40cm \times 17,25cm - (17,25cm)^2 + 17,25cm \times 40cm}{2 \times (17,25cm + 17,25cm)}$$

$$e_2^* = 15,69cm$$

$$Msd_2^* = Fsd.e_2^* = 159,08KN \times 15,69cm = 2495,97KN.cm$$

$$Msd_{2_{fim}} \geq \begin{cases} 0 \\ Msd_2 - Msd_2^* = 3434,2KN.cm - 2495,97KN.cm = 938,23KN.cm \end{cases}$$

$$Msd_{2_{fim}} = 932,23KN.cm$$

$$\frac{C_2}{C_1} = \frac{40cm}{40cm} = 1 :: K_2 = 0,60$$

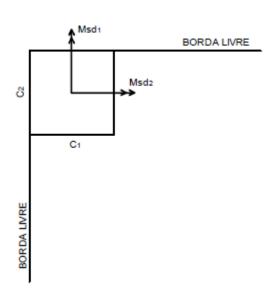
$$Tsd_2 = \frac{Fsd}{u^*.d} + K_2. \frac{Msd_{2_{fim}}}{wp_2.d} = \frac{159,08KN}{34,5cm \times 11,5cm} + 0,6 \times \frac{938,23KN.cm}{1200cm^2 \times 11,5cm}$$

$$Tsd_2 = 0,442 \frac{KN}{cm^2} = 4,42MPa$$

$$Trd_2 = 4,34MPa$$

$$Tsd_2 = 4,42MPa > Trd_2 = 4,34MPa : n\~{a}o ok!$$

Obs: Ao verificar o contorno C, chega-se à conclusão que será necessário mudar algo na estrutura para atender à condição proposta. Vou aumentar a seção dos pilares da estrutura para 50cm x 50cm.



3 - Dados:

$$C_1 = 50cm$$

$$C_2 = 50cm$$

$$d = h - d' = 16cm - 4.5cm = 11.5cm$$

$$Fsd = Fz \times 1,4 = 113,63KN \times 1,4 = 159,08KN (Compressão)$$

$$Msd_1 = M_y \times 1.4 = 1806KN.cm \times 1.4 = 2528.4KN.cm$$

$$Msd_2 = M_x \times 1,4 = 2453KN.cm \times 1,4 = 3434,2KN.cm$$

$$a_{01} \leq \begin{cases} 1.5 \times d = 1.5 \times 11.5cm = 17.25cm \\ C_1 = 50cm \end{cases} :: a_{01} = 17.25cm$$

$$a_{02} \leq \begin{cases} 1.5 \times d = 1.5 \times 11.5cm = 17.25cm \\ C_2 = 50cm \end{cases} :: a_{02} = 17.25cm$$

$$a_1 \leq \begin{cases} 1.5 \times d = 1.5 \times 11.5cm = 17.25cm \\ 0.5 \times C_1 = 0.5 \times 50cm = 25cm \end{cases} \div a_1 = 17.25cm$$

$$a_2 \leq \begin{cases} 1.5 \times d = 1.5 \times 11.5cm = 17.25cm \\ 0.5 \times C_2 = 0.5 \times 50cm = 25cm \end{cases} \therefore a_2 = 17.25cm$$

4 – Verificação do Contorno C:

$$u^* = a_{01} + a_{02} = 17,25cm + 17,25cm = 34,5cm$$

4.1 − Análise de Msd₁:

$$wp_1 = \frac{{C_1}^2}{4} + \frac{{C_1}.{C_2}}{2} = \frac{(50cm)^2}{4} + \frac{50cm \times 50cm}{2} = 1875cm^2$$

$$e_1^* = \frac{C_1 \cdot a_1 - a_1^2 + a_2 \cdot C_1}{2 \cdot (a_1 + a_2)} = \frac{50cm \times 17,25cm - (17,25cm)^2 + 17,25cm \times 50cm}{2 \times (17,25cm + 17,25cm)}$$

$$e_1^* = 20,69cm$$

$$Msd_1^* = Fsd.e_1^* = 159,08KN \times 20,69cm = 3291,37KN.cm$$

$$Msd_{1_{fim}} \geq \begin{cases} 0 \\ Msd_{1} - Msd_{1}^{*} = 2528,4KN.cm - 3291,37KN.cm = -762,97KN.cm \end{cases}$$

$$Msd_{1_{fim}} = 0$$

$$\frac{C_1}{C_2} = \frac{50cm}{50cm} = 1 :: K_1 = 0,60$$

$$Tsd_1 = \frac{Fsd}{u^*.d} + K_1.\frac{Msd_{1_{fim}}}{wp_1.d} = \frac{159,08KN}{34,5cm \times 11,5cm} + 0,60 \times \frac{0}{1875cm^2 \times 11,5cm}$$

$$Tsd_1 = 0.402 \frac{KN}{cm^2} = 4.02MPa$$

$$Trd_2 = 4,34MPa$$

$$Tsd_1 = 4.02MPa < Trd_2 = 4.34MPa : ok!$$

4.2 – Análise de Msd₂:

$$wp_2 = \frac{{C_2}^2}{4} + \frac{C_2.\,C_1}{2} = \frac{(50cm)^2}{4} + \frac{50cm \times 50cm}{2} = 1875cm^2$$

$$e_2^* = \frac{C_2 \cdot a_2 - a_2^2 + a_1 \cdot C_2}{2 \cdot (a_2 + a_1)} = \frac{50cm \times 17,25cm - (17,25cm)^2 + 17,25cm \times 50cm}{2 \times (17,25cm + 17,25cm)}$$

$$e_2^* = 20,69cm$$

$$Msd_2^* = Fsd.e_2^* = 159,08KN \times 20,69cm = 3291,37KN.cm$$

$$Msd_{2_{fim}} \geq \begin{cases} 0 \\ Msd_2 - Ms{d_2}^* = 3434,2KN.\,cm - 3291,37KN.\,cm = 142,83KN.\,cm \end{cases}$$

$$Msd_{2_{fim}} = 142,83KN.cm$$

$$\frac{C_2}{C_1} = \frac{50cm}{50cm} = 1 : K_2 = 0.60$$

$$Tsd_2 = \frac{Fsd}{u^*.d} + K_2. \frac{Msd_{2_{fim}}}{wp_2.d} = \frac{159,08KN}{34,5cm \times 11,5cm} + 0.6 \times \frac{142,83KN.cm}{1875cm^2 \times 11,5cm}$$

$$Tsd_2 = 0.405 \frac{KN}{cm^2} = 4.05MPa$$

$$Trd_2 = 4,34MPa$$

$$Tsd_2 = 4.05MPa < Trd_2 = 4.34MPa : ok!$$

5 – Verificação do Contorno C':

$$u^* = a_1 + a_2 + \pi.d = 17,25cm + 17,25cm + \pi \times 11,5cm = 70,63cm$$

5.1 – Análise de Msd₁:

$$wp_1 = \frac{{C_1}^2}{4} + \frac{{C_1}.{C_2}}{2} + 2.\,{C_2}.\,d + 4.\,d^2 + \frac{\pi.\,d.\,{C_1}}{2}$$

$$= \frac{(50cm)^2}{4} + \frac{50cm \times 50cm}{2} + 2 \times 50cm \times 11,5cm + 4 \times (11,5cm)^2 = 3554cm^2$$

$$= \frac{\pi \times 11,5cm \times 50cm}{2} = 903,21cm^2$$

$$wp_1 = 3554cm^2 + 903,21cm^2 = 4457,21cm^2$$

$$e_1^* = \frac{C_1.\,a_1 - a_1^2 + a_2.\,C_1 + 4.\,a_2.\,d + 8.\,d^2 + \pi.\,d.\,C_1}{2.\,(a_1 + a_2 + \pi.\,d)}$$

$$= 50cm \times 17,25cm - (17,25cm)^2 + 17,25cm \times 50cm = 1427,44cm^2$$

$$= 4 \times 17,25cm \times 11,5cm + 8 \times (11,5cm)^2 + \pi \times 11,5cm \times 50cm = 3657,92cm^2$$

$$= 2 \times (17,25cm + 17,25cm + \pi \times 11,5cm) = 141,26cm$$

$$e_1^* = \frac{1427,44cm^2 + 3657,92cm^2}{141,26cm} = 36cm$$

$$Msd_1^* = Fsd.e_1^* = 159,08KN \times 36cm = 5726,88KN.cm$$

$$Msd_{1fim} \geq \begin{cases} 0 \\ Msd_{1} - Msd_{1}^{*} = 2528,4KN.cm - 5726,88KN.cm = -3198,48KN.cm \end{cases}$$

$$Msd_{1_{fim}} = 0$$

$$\frac{C_1}{C_2} = \frac{50cm}{50cm} = 1 :: K_1 = 0.60$$

$$Tsd_1 = \frac{Fsd}{u^*.d} + K_1.\frac{Msd_{1_{fim}}}{wp_1.d} = \frac{159,08KN}{70,63cm \times 11,5cm} + 0,60 \times \frac{0}{4457,21cm^2 \times 11,5cm}$$

$$Tsd_1 = 0.20 \, KN/cm^2 = 2.00 MPa$$

Ø10mm à cada 14cm :
$$\rho_x = \frac{As_x}{Ac} = \frac{0.8cm^2}{14cm \times 16cm} = 0.0035$$

Ø12.5mm à cada 13cm :
$$\rho_y = \frac{As_y}{Ac} = \frac{1,25cm^2}{14cm \times 16cm} = 0,0059$$

$$\rho = \sqrt{\rho_x \cdot \rho_y} = \sqrt{0.0035 \times 0.0059} = 0.0045$$

$$Trd_1 = 0.13.\left(1 + \sqrt{\frac{20}{d}}\right).\left(100.\rho.fck\right)^{\frac{1}{3}}$$

$$Trd_1 = 0.13 \times \left(1 + \sqrt{\frac{20}{11.5cm}}\right) \times (100 \times 0.0045 \times 25MPa)^{\frac{1}{3}} = 0.68MPa$$

$$Tsd_1 = 2,00MPa > Trd_1 = 0,68MPa : Armar à punção!$$

5.1.1 – Cálculo da armadura de punção:

$$\alpha = 90^{\circ} : sen 90^{\circ} = 1.0$$

$$s_r \leq 0.75.d : s_r \leq 0.75 \times 11.5cm : s_r \leq 8.63cm : s_r = 8.0cm$$

$$fywd = 250 + 185.\frac{(h-15)}{20} = 250 + 185 \times \frac{(16cm-15)}{20} = 259,20 MPa \div 26 \frac{KN}{cm^2}$$

$$Trd_3 = 0.10.\left(1+\sqrt{\frac{20}{d}}\right).\left(100.\rho.fck\right)^{\frac{1}{3}} + 1.5.\frac{d}{s_r}.\frac{Asw.fywd.sen \propto u.d}{u.d}$$

=
$$0.10 \times \left(1 + \sqrt{\frac{20}{11.5cm}}\right) \times (100 \times 0.0045 \times 25MPa)^{\frac{1}{5}} = 0.52MPa$$

$$= 1.5 \times \frac{11.5cm}{8cm} \times \frac{Asw \times 259.2MPa \times 1}{70.63cm \times 11.5cm} = 0.69 \times Asw \times \frac{MPa}{cm^2}$$

$$Trd_3 = 0.52MPa + 0.69 \times Asw \times \frac{MPa}{cm^2}$$

$$Tsd_1 \leq Trd_3$$

$$2MPa \le 0.52MPa + 0.69 \times Asw \times \frac{MPa}{cm^2}$$

$$Asw_1 \ge 2,14cm^2$$

5.2 – Análise de Msd₂:

$$wp_2 = \frac{{C_2}^2}{4} + \frac{{C_2}.\,{C_1}}{2} + 2.\,{C_1}.\,d + 4.\,d^2 + \frac{\pi.d.\,{C_2}}{2}$$

$$= \frac{(50cm)^2}{4} + \frac{50cm \times 50cm}{2} + 2 \times 50cm \times 11,5cm + 4 \times (11,5cm)^2 = 3554cm^2$$

$$= \frac{\pi \times 11,5cm \times 50cm}{2} = 903,21cm^2$$

$$wp_2 = 3554cm^2 + 903,21cm^2 = 4457,21cm^2$$

$$e_2^* = \frac{C_2 \cdot a_2 - a_2^2 + a_1 \cdot C_2 + 4 \cdot a_1 \cdot d + 8 \cdot d^2 + \pi \cdot d \cdot C_2}{2 \cdot (a_2 + a_1 + \pi \cdot d)}$$

$$= 50cm \times 17,25cm - (17,25cm)^2 + 17,25cm \times 50cm = 1427,44cm^2$$

$$= 4 \times 17,25cm \times 11,5cm + 8 \times (11,5cm)^2 + \pi \times 11,5cm \times 50cm = 3657,92cm^2$$

$$= 2 \times (17,25cm + 17,25cm + \pi \times 11,5cm) = 141,26cm$$

$${e_2}^* = \frac{1427,44cm^2 + 3657,92cm^2}{141,26cm} = 36\text{cm}$$

$$Msd_2^* = Fsd.e_2^* = 159,08KN \times 36cm = 5726,88KN.cm$$

$$Msd_{2fim} \geq \begin{cases} 0 \\ Msd_2 - Msd_2^* = 3434,2KN.cm - 5726,88KN.cm = -2292,68KN.cm \end{cases}$$

$$Msd_{2fim} = 0$$

$$\frac{C_2}{C_1} = \frac{50cm}{50cm} = 1 : K_2 = 0.60$$

$$Tsd_2 = \frac{Fsd}{u^*.d} + K_2. \frac{Msd_{2_{fim}}}{wp_2.d} = \frac{159,08KN}{70,63cm \times 11,5cm} + 0,60 \times \frac{0}{4457,21cm^2 \times 11,5cm}$$

$$Tsd_2 = 0.20 \, KN/cm^2 = 2.00 MPa$$

$$Trd_1 = 0.68MPa$$

$$Tsd_2 = 2,00MPa > Trd_1 = 0,68MPa : Armar à punção!$$

5.2.1 – Cálculo da armadura de punção:

$$\alpha = 90^{\circ} : sen 90^{\circ} = 1.0$$

$$s_r \le 0.75.d : s_r \le 0.75 \times 11.5cm : s_r \le 8.63cm : s_r = 8.0cm$$

$$fywd = 250 + 185.\frac{(h-15)}{20} = 250 + 185 \times \frac{(16cm-15)}{20} = 259,20 MPa \div 26 \frac{KN}{cm^2}$$

$$Trd_{3} = 0.10. \left(1 + \sqrt{\frac{20}{d}}\right). (100.\rho.fck)^{\frac{1}{3}} + 1.5. \frac{d}{s_{r}}. \frac{Asw.fywd.sen}{u.d} \propto 0.10. \left(1 + \sqrt{\frac{20}{d}}\right). (100.\rho.fck)^{\frac{1}{3}} + 1.5. \frac{d}{s_{r}}. \frac{Asw.fywd.sen}{u.d} \propto 0.10. \left(1 + \sqrt{\frac{20}{d}}\right). (100.\rho.fck)^{\frac{1}{3}} + 1.5. \frac{d}{s_{r}}. \frac{Asw.fywd.sen}{u.d} \propto 0.10. \left(1 + \sqrt{\frac{20}{d}}\right). (100.\rho.fck)^{\frac{1}{3}} + 1.5. \frac{d}{s_{r}}. \frac{Asw.fywd.sen}{u.d} \propto 0.10. \left(1 + \sqrt{\frac{20}{d}}\right). (100.\rho.fck)^{\frac{1}{3}} + 1.5. \frac{d}{s_{r}}. \frac{Asw.fywd.sen}{u.d} \propto 0.10. \left(1 + \sqrt{\frac{20}{d}}\right). (100.\rho.fck)^{\frac{1}{3}} + 1.5. \frac{d}{s_{r}}. \frac{Asw.fywd.sen}{u.d} \propto 0.10. \left(1 + \sqrt{\frac{20}{d}}\right). (100.\rho.fck)^{\frac{1}{3}} + 1.5. \frac{d}{s_{r}}. \frac{Asw.fywd.sen}{u.d} \propto 0.10. \left(1 + \sqrt{\frac{20}{d}}\right). (100.\rho.fck)^{\frac{1}{3}} + 1.5. \frac{d}{s_{r}}. \frac{Asw.fywd.sen}{u.d} \propto 0.10. \left(1 + \sqrt{\frac{20}{d}}\right). (100.\rho.fck)^{\frac{1}{3}} + 1.5. \frac{d}{s_{r}}. \frac{Asw.fywd.sen}{u.d} \propto 0.10. \left(1 + \sqrt{\frac{20}{d}}\right). (100.\rho.fck)^{\frac{1}{3}} + 1.5. \frac{d}{s_{r}}. \frac{Asw.fywd.sen}{u.d} \propto 0.10. \left(1 + \sqrt{\frac{20}{d}}\right). (100.\rho.fck)^{\frac{1}{3}} + 1.5. \frac{d}{s_{r}}. \frac{Asw.fywd.sen}{u.d} \propto 0.10. \left(1 + \sqrt{\frac{20}{d}}\right). (100.\rho.fck)^{\frac{1}{3}} + 1.5. \frac{d}{s_{r}}. \frac{Asw.fywd.sen}{u.d} \propto 0.10. \left(1 + \sqrt{\frac{20}{d}}\right). (100.\rho.fck)^{\frac{1}{3}} + 1.5. \frac{d}{s_{r}}. \frac{Asw.fywd.sen}{u.d} \propto 0.10. \left(1 + \sqrt{\frac{20}{d}}\right). (100.\rho.fck)^{\frac{1}{3}} + 1.5. \frac{d}{s_{r}}. \frac{Asw.fywd.sen}{u.d} \sim 0.10. \left(1 + \sqrt{\frac{20}{d}}\right). (100.\rho.fck)^{\frac{1}{3}} + 1.5. \frac{d}{s_{r}}. \frac{Asw.fywd.sen}{u.d} \sim 0.10. \left(1 + \sqrt{\frac{20}{d}}\right). (100.\rho.fck)^{\frac{1}{3}} + 1.5. \frac{d}{s_{r}}. \frac{Asw.fywd.sen}{u.d} \sim 0.10. \left(1 + \sqrt{\frac{20}{d}}\right). (100.\rho.fck)^{\frac{1}{3}} + 1.5. \frac{d}{s_{r}}. \frac{Asw.fywd.sen}{u.d} \sim 0.10. \left(1 + \sqrt{\frac{20}{d}}\right). (100.\rho.fck)^{\frac{1}{3}} + 1.5. \frac{d}{s_{r}}. \frac{Asw.fywd.sen}{u.d} \sim 0.10. \left(1 + \sqrt{\frac{20}{d}}\right). (100.\rho.fck)^{\frac{1}{3}} + 1.5. \frac{d}{s_{r}}. \frac{Asw.fywd.sen}{u.d} \sim 0.10. \left(1 + \sqrt{\frac{20}{d}}\right). (100.\rho.fck)^{\frac{1}{3}} + 1.5. \frac{d}{s_{r}}. \frac{Asw.fywd.sen}{u.d} \sim 0.10. \left(1 + \sqrt{\frac{20}{d}}\right). (100.\rho.fck)^{\frac{1}{3}} + 1.0. \left(1 + \sqrt{\frac{20}{d}}\right). (100.\rho.fck)^{\frac{1}{3}} + 1.0. \left(1 + \sqrt{\frac{20}{d}}\right). (100.\rho.fck)^{\frac{1}{3}} + 1.0. \left(1 + \sqrt{\frac{20}{d}}\right). (100.\rho.fck)^{\frac{1}$$

= 0,10 ×
$$\left(1 + \sqrt{\frac{20}{11,5cm}}\right)$$
 × $\left(100 \times 0,0045 \times 25MPa\right)^{\frac{1}{8}}$ =0,52MPa

$$=1.5\times\frac{11.5cm}{8cm}\times\frac{Asw\times259.2MPa\times1}{70.63cm\times11.5cm}=0.69\times Asw\times\frac{MPa}{cm^2}$$

$$Trd_3 = 0.52MPa + 0.69 \times Asw \times \frac{MPa}{cm^2}$$

$$Tsd_2 \leq Trd_3$$

$$2MPa \le 0.52MPa + 0.69 \times Asw \times \frac{MPa}{cm^2}$$

$$Asw_2 \ge 2,14cm^2$$

5.3 – Armadura de punção total calculada:

$$Asw \geq Asw_1 + Asw_2 \ \therefore \ Asw \geq 2,14cm^2 + 2,14cm^2 \ \therefore \ Asw \geq 4,28cm^2$$

5.4 – Cálculo da armadura de punção obrigatória:

$$Asw \ge \frac{1}{2} \cdot \frac{Fsd}{fywd} : Asw \ge \frac{1}{2} \times \frac{159,08KN}{26\frac{KN}{cm^2}} : Asw \ge 3,06cm^2$$

Serão no mínimo 3 contornos de armação à punção, então:

$$Asw \ge \frac{3,06cm^2}{3\ contornos} = 1,02cm^2$$

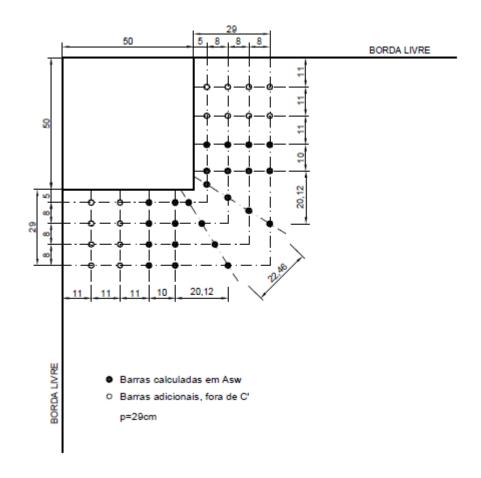
$$Asw : 4,28cm^2 > 1,02cm^2 : ok!$$

5.5 – Disposição construtiva:

$$s_0 \leq 0.5.\,d \, \div s_0 \leq 0.5 \times 11.5cm \, \div s_0 \leq 5.75cm \, \div s_0 = 5.00cm$$

$$s_r \leq 0.75.d : s_r \leq 0.75 \times 11.5cm : s_r \leq 8.63cm : s_r = 8.00cm$$

$$s_e \le 2.d : s_e \le 2 \times 11,5cm : s_e \le 23cm : s_e = 23,00cm$$



 $6010.0mm = 4.8cm^2 > 4.28cm^2 : ok! : Por contorno serão 10010.0mm$

6 – Verificação do Contorno C":

$$p = 29cm$$

$$u^* = a_1 + a_2 + \pi.d + \frac{\pi.p}{2} = 17,25cm + 17,25cm + \pi \times 11,5cm + \frac{\pi \times 29cm}{2}$$

$$u^* = 116,18cm$$

6.1 – Análise de Msd₁:

$$\begin{split} wp_1 &= \frac{{C_1}^2}{4} + \frac{{C_1}.{C_2}}{2} + 2.\,{C_2}.\,d + 4.\,d^2 - \frac{\pi.\,d.\,{C_1}}{2} + {C_2}.\,p + 4.\,d.\,p + \frac{\pi.\,p.\,{C_1}}{4} + p^2 \\ &= \frac{(50cm)^2}{4} + \frac{50cm \times 50cm}{2} + 2 \times 50cm \times 11,5cm + 4 \times (11,5cm)^2 = 3554cm^2 \end{split}$$

$$= \frac{\pi \times 11,5cm \times 50cm}{2} = 903,21cm^2$$

$$=50cm \times 29cm + 4 \times 11,5cm \times 29cm + \frac{\pi \times 29cm \times 50cm}{4} + (29cm)^2 = 4763,83cm^2$$

$$wp_1 = 3554cm^2 - 903,21cm^2 + 4763,83cm^2 = 7414,62cm^2$$

$$e_1{}^* = \frac{{}^{C_1.a_1-a_1}{}^2 + a_2.C_1 + 4.a_2.d + 8.d^2 + \pi.d.C_1 + 2.a_2.p + 8.d.p + \frac{\pi.p.C_1}{2} + 2.p^2}{2.\left(a_1 + a_2 + \pi.d + \frac{\pi.p}{2}\right)}$$

$$= 50cm \times 17,25cm - (17,25cm)^2 + 17,25cm \times 50cm = 1427,44cm^2$$

$$= 4 \times 17,25cm \times 11,5cm + 8 \times (11,5cm)^2 + \pi \times 11,5cm \times 50cm = 3657,92cm^2$$

$$= 2 \times 17,25cm \times 29cm + 8 \times 11,5cm \times 29cm = 3668,5cm^{2}$$

$$= \frac{\pi \times 29 \text{cm} \times 50 \text{cm}}{2} + 2 \times (29 \text{cm})^2 = 3959,65 \text{cm}^2$$

$$=2\times \left(17,25cm+17,25cm+\pi\times 11,5cm+\frac{\pi\times 29cm}{2}\right)=232,36cm$$

$${e_1}^* = \frac{1427,\!44cm^2 + 3657,\!92cm^2 + 3668,\!5cm^2 + 3959,\!65cm^2}{232,\!36cm} = 54,\!71\mathrm{cm}$$

$$Msd_1^* = Fsd.e_1^* = 159,08KN \times 54,71cm = 8703,27KN.cm$$

$$Msd_{1fim} \geq \begin{cases} 0 \\ Msd_{1} - Msd_{1}^{*} = 2528,4KN.cm - 8703,27KN.cm = -6174,87KN.cm \end{cases}$$

$$Msd_{1fim} = 0$$

$$\frac{C_1}{C_2} = \frac{50cm}{50cm} = 1 : K_1 = 0.60$$

$$Tsd_1 = \frac{Fsd}{u^*.d} + K_1.\frac{Msd_{1_{fim}}}{wp_1.d} = \frac{159,08KN}{116,18cm \times 11,5cm} + 0,60 \times \frac{0}{7414,62cm^2 \times 11,5cm}$$

$$Tsd_1 = 0.119 \frac{KN}{cm^2} = 1.19MPa$$

$$Trd_1 = 0,68MPa$$

 $Tsd_1 = 1,19MPa > Trd_1 = 0,68MPa : Colocar mais um contorno de armação!$

7 – Inserir mais um contorno de armação:

Obs: Como o momento final é nulo e continuará sendo nulo com o aumento da excentricidade, consegue-se estimar o valor de "p" para que a solicitação seja menor que a resistência.

$$Tsd_1 \leq Trd_1 : Tsd_1 \leq 0,68MPa : Tsd_1 \leq 0,068 \frac{KN}{cm^2}$$

$$Tsd_1 = \frac{Fsd}{u^*.d} + 0 = \frac{159,08KN}{u^*.11,5cm} = \frac{13,83}{u^*} \frac{KN}{cm}$$

$$\frac{13,\!83}{u^*} \frac{KN}{cm} \leq 0,\!068 \frac{KN}{cm^2} :: u^* \geq 204cm$$

$$u^* = a_1 + a_2 + \pi.d + \frac{\pi.p}{2} = 17,25cm \times 2 + \pi \times 11,5cm + \frac{\pi \times p}{2}$$

$$u^* = 70,63cm + 1,57 \times p$$

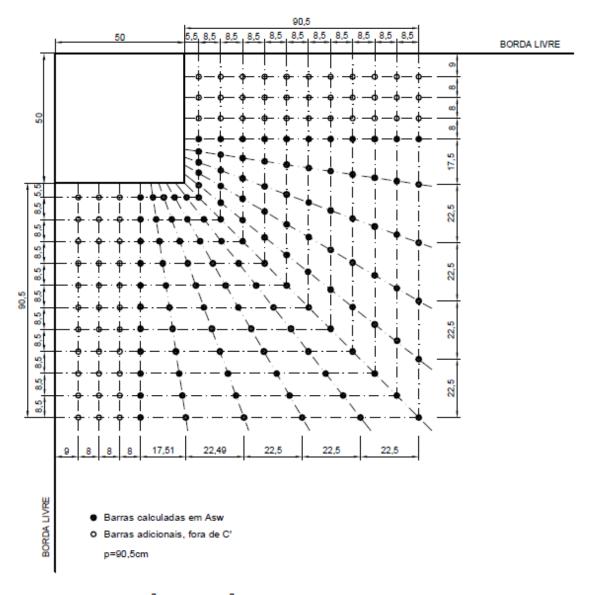
$$70,63cm + 1,57 \times p \ge 204cm : p \ge 84,95cm$$

8 – Nova disposição construtiva:

$$s_0 \le 0.5.d : s_0 \le 0.5 \times 11.5cm : s_0 \le 5.75cm : s_0 = 5.50cm$$

$$s_r \leq 0.75.d : s_r \leq 0.75 \times 11.5cm : s_r \leq 8.63cm : s_r = 8.50cm$$

$$s_e \leq 2.\,d :: s_e \leq 2 \times 11,5 cm :: s_e \leq 23 cm :: s_e = 23,00 cm$$



 $1108.0mm = 5.5cm^2 > 4.28cm^2 : ok! : Por contorno serão 1708.0mm$

9 – Verificação do novo contorno C":

$$p = 90,5cm$$

$$u^* = a_1 + a_2 + \pi.d + \frac{\pi.p}{2} = 17,25cm + 17,25cm + \pi.11,5cm + \frac{\pi.90,5cm}{2}$$

$$u^* = 212,79cm$$

9.1 – Análise de Msd₁:

$$wp_1 = \frac{{C_1}^2}{4} + \frac{{C_1}.{C_2}}{2} + 2.\,{C_2}.\,d + 4.\,d^2 - \frac{\pi.\,d.\,{C_1}}{2} + {C_2}.\,p + 4.\,d.\,p + \frac{\pi.\,p.\,{C_1}}{4} + p^2$$

$$= \frac{(50cm)^2}{4} + \frac{50cm \times 50cm}{2} + 2 \times 50cm \times 11,5cm + 4 \times (11,5cm)^2 = 3554cm^2$$

$$= \frac{\pi \times 11,5cm \times 50cm}{2} = 903,21cm^2$$

$$= 50cm \times 90,5cm + 4 \times 11,5cm \times 90,5cm + \frac{\pi \times 90,5cm \times 50cm}{4} + (90,5cm)^2 = 20432,18cm^2$$

$$= wp_1 = 3554cm^2 - 903,21cm^2 + 20432,18cm^2 = 23082,97cm^2$$

$$= e_1^* = \frac{c_1.a_1-a_1^2+a_2.c_1+4.a_2.d_1+8.d_2^2+\pi.d_1C_2+2.a_2.p_1+8.d.p_1+\frac{\pi.p.C_1}{2}+2.p^2 }{2.(a_1+a_2+\pi.d_1+\frac{\pi.p.}{2})}$$

$$= 50cm \times 17,25cm - (17,25cm)^2 + 17,25cm \times 50cm = 1427,44cm^2$$

$$= 4 \times 17,25cm \times 11,5cm + 8 \times (11,5cm)^2 + \pi \times 11,5cm \times 50cm = 3657,92cm^2$$

$$= 2 \times 17,25cm \times 90,5cm + 8 \times 11,5cm \times 90,5cm = 11448,25cm^2$$

$$= \frac{\pi \times 90,5cm \times 50cm}{2} + 2 \times (90,5cm)^2 = 23488,35cm^2$$

$$= 2 \times \left(17,25cm + 17,25cm + \pi \times 11,5cm + \frac{\pi \times 90,5cm}{2}\right) = 425,57cm$$

$$= e_1^* = \frac{1427,44cm^2 + 3657,92cm^2 + 11448,25cm^2 + 23488,35cm^2}{425,57cm} = 94,04cm$$

$$Msd_1^* = Fsd.e_1^* = 159,08KN \times 94,04cm = 14959,88KN.cm$$

$$Msd_1_{fim} = 0$$

$$\frac{C_1}{C_2} = \frac{50cm}{50cm} = 1 \therefore K_1 = 0,60$$

$$Tsd_1 = \frac{Fsd}{100cm} + K_1. \frac{Msd_1_{fim}}{mp_1.d} = \frac{159,08KN}{212,79cm \times 11,5cm} + 0,60 \times \frac{0}{23082,97cm^2 \times 11,5cm}$$

$$Tsd_1 = \frac{Fsd}{u^*.d} + K_1. \frac{Msd_1_{fim}}{mp_1.d} = \frac{159,08KN}{212,79cm \times 11,5cm} + 0 = 0,065 \frac{KN}{cm^2} = 0,65MPa$$

$$Trd_1 = 0,68MPa$$

 $Tsd_1 = 0.65MPa < Trd_1 = 0.68MPa : ok!$

9.2 – Análise de Msd₂:

 $Tsd_2 = 0.065 \frac{KN}{cm^2} = 0.65MPa$

$$\begin{split} wp_2 &= \frac{C_1^2}{4} + \frac{C_2 \cdot C_1}{2} + 2 \cdot C_1 \cdot d + 4 \cdot d^2 - \frac{\pi \cdot d \cdot C_2}{2} + C_1 \cdot p + 4 \cdot d \cdot p + \frac{\pi \cdot p \cdot C_2}{4} + p^2 \\ &= \frac{(50cm)^2}{4} + \frac{50cm \times 50cm}{2} + 2 \times 50cm \times 11.5cm + 4 \times (11.5cm)^2 = 3554cm^2 \\ &= \frac{\pi \times 11.5cm \times 50cm}{2} = 903.21cm^2 \\ &= 50cm \times 90.5cm + 4 \times 11.5cm \times 90.5cm + \frac{\pi \times 90.5cm \times 50cm}{4} + (90.5cm)^2 = 20432.18cm^2 \\ wp_2 &= 3554cm^2 - 903.21cm^2 + 20432.18cm^2 = 23082.97cm^2 \\ e_2^* &= \frac{c_2 \cdot a_2 - a_2^2 + a_1 \cdot c_2 + 4 \cdot a_1 \cdot d + 8 \cdot d^2 + \pi \cdot d \cdot c_2 + 2 \cdot a_1 \cdot p + 8 \cdot d \cdot p + \frac{\pi \cdot p \cdot C_2}{2} + 2 \cdot p^2}{2 \cdot (a_2 + a_1 + \pi \cdot d + \frac{\pi \cdot p}{2})} \\ &= 50cm \times 17.25cm - (17.25cm)^2 + 17.25cm \times 50cm = 1427.44cm^2 \\ &= 4 \times 17.25cm \times 11.5cm + 8 \times (11.5cm)^2 + \pi \times 11.5cm \times 50cm = 3657.92cm^2 \\ &= 2 \times 17.25cm \times 90.5cm + 8 \times 11.5cm \times 90.5cm = 11448.25cm^2 \\ &= \frac{\pi \times 90.5cm \times 50cm}{2} + 2 \times (90.5cm)^2 = 23488.35cm^2 \\ &= 2 \times \left(17.25cm + 17.25cm + \pi \times 11.5cm + \frac{\pi \times 90.5cm}{2}\right) = 425.57cm \\ &e_2^* &= \frac{1427.44cm^2 + 3657.92cm^2 + 11448.25cm^2 + 23488.35cm^2}{425.57cm} = 94.04cm \\ Msd_2_{fim} &= 0 \\ Msd_2_{fim} &= 0 \\ \frac{C_2}{C_1} &= \frac{50cm}{50cm} = 1 \cdot K_2 = 0.60 \\ Tsd_2 &= \frac{Fsd}{u^*.d} + K_2 \cdot \frac{Msd_2_{fim}}{wp_2.d} = \frac{159.08KN}{212.79cm \times 11.5cm} + 0.60 \times \frac{0}{23082.97cm^2 \times 11.5cm} \\ Tsd_2 &= \frac{Fsd}{u^*.d} + K_2 \cdot \frac{Msd_2_{fim}}{wp_2.d} = \frac{159.08KN}{212.79cm \times 11.5cm} + 0.60 \times \frac{0}{23082.97cm^2 \times 11.5cm} \\ \end{array}$$

$$Trd_1 = 0.68MPa$$

$$Tsd_1 = 0.65MPa < Trd_1 = 0.68MPa : ok!$$

Verificação dos pilares P2, P8, usando como referência o pilar P2:

BORDA LIVRE



$$C_1 = 50cm$$

$$C_2 = 50cm$$

$$d = h - d' = 16cm - 4,5cm = 11,5cm$$

$$Fsd = Fz. \, 1,4 = 170,36KN \times 1,4 = 238,50KN \, (Compressão)$$

$$Msd_1 = M_x.1,4 = 3811KN.cm \times 1,4 = 5335,4KN.cm$$

$$Msd_2 = M_v. 1,4 = 0 \ KN. \ cm \times 1,4 = 0 \ KN. \ cm$$

$$a_0 \leq 1,\!5.\,d$$
 .: $a_0 \leq 1,\!5 \times 11,\!5cm$.: $a_0 \leq 17,\!25cm$.: $a_0 = 17,\!25cm$

$$a \leq \begin{cases} 1, 5.d = 1, 5 \times 11, 5cm = 17, 25cm \\ 0, 5. C_2 = 0, 5 \times 50cm = 25cm \end{cases} :: a = 17, 25cm$$

2 – Verificação do Contorno C:

$$u^* = 2.a_0 + C_2 = 2 \times 17,25cm + 50cm = 84,5cm$$

$$wp_1 = \frac{{C_1}^2}{2} + \frac{{C_1}.{C_2}}{2} = \frac{(50cm)^2}{2} + \frac{50cm \times 50cm}{2} = 2500cm^2$$

$$wp_2 = \frac{{C_2}^2}{4} + C_1$$
. $C_2 = \frac{(50cm)^2}{4} + 50cm \times 50cm = 3125cm^2$

$$e^* = \frac{C_1 \cdot a_0 - {a_0}^2 + \frac{C_1 \cdot C_2}{2}}{2 \cdot a_0 + C_2} = \frac{50cm \times 17,25cm - (17,25cm)^2 + \frac{50cm \times 50cm}{2}}{2 \times 17,25cm + 50cm}$$

$$e^* = 21,48cm$$

$$Msd_1^* = Fsd.e^* = 238,5KN \times 21,48cm = 5122,98KN.cm$$

$$Msd_{1_{fim}} \ge \begin{cases} 0 \\ Msd_{1} - Msd_{1}^{*} = 5335,4KN.cm - 5122,98KN.cm = 212,42KN.cm \end{cases}$$

$$Msd_{1_{fim}} = 212,42KN.cm$$

$$\frac{C_1}{C_2} = \frac{50cm}{50cm} = 1 : K_1 = 0,60$$

$$\frac{C_2}{2.\,C_1} = \frac{50\,cm}{2\times50\,cm} = 0.5 \, \therefore K_2 = 0.45$$

$$Tsd = \frac{Fsd}{u^*.d} + K_1.\frac{Msd_{1_{fim}}}{wp_1.d} + K_2.\frac{Msd_2}{wp_2.d}$$

$$Tsd = \frac{238,50KN}{84,5cm \times 11,5cm} + 0.6 \times \frac{212,42KN.cm}{2500cm^2 \times 11,5cm} + 0.45 \times \frac{0}{3125cm^2 \times 11,5cm}$$

$$Tsd = 0.25 \frac{KN}{cm^2} = 2.5MPa$$

$$Trd_2 = 0.27. \propto_v. fcd = 0.27 \times \left(1 - \frac{fck}{250}\right) \times \left(\frac{fck}{1.4}\right) = 0.27 \times \left(1 - \frac{25MPa}{250}\right) \times \left(\frac{25MPa}{1.4}\right)$$

$$Trd_2 = 4,34MPa$$

$$Tsd = 2.5MPa < Trd_2 = 4.34MPa : ok!$$

2 – Verificação do Contorno C':

$$u^* = 2.a + C_2 + 2.\pi.d = 2 \times 17,\!25cm + 50cm + 2 \times \pi \times 11,\!5cm = 156,\!76cm$$

$$wp_1 = \frac{{C_1}^2}{2} + \frac{{C_1 \cdot C_2}}{2} + 2 \cdot C_2 \cdot d + 8 \cdot d^2 + \pi \cdot d \cdot C_1$$
$$= \frac{(50cm)^2}{2} + \frac{50cm \times 50cm}{2} + 2 \times 50cm \times 11,5cm = 3650cm^2$$

$$= 8 \times (11,5cm)^2 + \pi \times 11,5cm \times 50cm = 2864,42cm^2$$

$$wp_1 = 3650cm^2 + 2864,42cm^2 = 6514,42cm^2$$

$$wp_2 = \frac{{C_2}^2}{4} + C_1.C_2 + 4.C_1.d + 8.d^2 + \pi.d.C_2$$

$$= \frac{(50cm)^2}{4} + 50cm \times 50cm + 4 \times 50cm \times 11,5cm = 5425cm^2$$

$$= 8 \times (11,5cm)^2 + \pi \times 11,5cm \times 50cm = 2864,42cm^2$$

$$wp_2 = 5425cm^2 + 2864,42cm^2 = 8289,42cm^2$$

$$e^* = \frac{C_1.a - a^2 + \frac{C_1 + C_2}{2} + 2.C_2.d + 8.d^2 + \pi.d.C_1}{2.a + C_2 + 2.\pi.d} =$$

$$= 50cm \times 17,25cm - (17,25cm)^2 + \frac{50cm + 50cm}{2} = 614,94cm^2$$

$$= 2 \times 50cm \times 11,5cm + 8 \times (11,5cm)^2 + \pi \times 11,5cm \times 50cm = 4014,42cm^2$$

$$= 2 \times 17,25cm + 50cm + 2 \times \pi \times 11,5cm = 156,76cm$$

$$e^* = \frac{614,94cm^2 + 4014,42cm^2}{156,76cm} = 29,53cm$$

$$Msd_1^* = Fsd.e^* = 238,5KN \times 29,53cm = 7042,91KN.cm$$

$$Msd_{1fim} \geq \begin{cases} 0 \\ Msd_{1} - Msd_{1}^{*} = 5335,\!4KN.\,cm - 7042,\!91KN.\,cm = -1707,\!51KN.\,cm \end{cases}$$

$$Msd_{1_{fim}} = 0$$

$$\frac{C_1}{C_2} = \frac{50cm}{50cm} = 1 :: K_1 = 0,60$$

$$\frac{C_2}{2.C_1} = \frac{50cm}{2 \times 50cm} = 0.5 : K_2 = 0.45$$

$$Tsd = \frac{Fsd}{u^*.d} + K_1.\frac{Msd_{1_{fim}}}{wp_1.d} + K_2.\frac{Msd_2}{wp_2.d}$$

$$Tsd = \frac{238,50KN}{156,76cm \times 11,5cm} + 0,6 \times \frac{0}{2500cm^2 \times 11,5cm} + 0,45 \times \frac{0}{3125cm^2 \times 11,5cm}$$

$$Tsd = 0.132 \frac{KN}{cm^2} = 1.32MPa$$

Ø12,5mm à cada 12cm :
$$\rho_x = \frac{As_x}{Ac} = \frac{1,25cm^2}{12cm \times 16cm} = 0,0065$$

Ø12.5mm à cada 13cm :
$$\rho_y = \frac{As_y}{Ac} = \frac{1,25cm^2}{13cm \times 16cm} = 0,006$$

$$\rho = \sqrt{\rho_x \cdot \rho_y} = \sqrt{0.0065 \times 0.006} = 0.0062$$

$$Trd_1 = 0.13. \left(1 + \sqrt{\frac{20}{d}}\right). (100.\rho.fck)^{\frac{1}{3}}$$

$$Trd_1 = 0.13 \times \left(1 + \sqrt{\frac{20}{11.5cm}}\right) \times (100 \times 0.0062 \times 25MPa)^{\frac{1}{3}} = 0.75MPa$$

 $Tsd = 1,32MPa > Trd_1 = 0,75MPa :: Armar à punção!$

2.1 – Cálculo da armadura de punção:

$$\alpha = 90^{\circ} : sen90^{\circ} = 1,0$$

$$s_r \leq 0.75.d :: s_r \leq 0.75 \times 11.5cm :: s_r \leq 8.63cm :: s_r = 8.0cm$$

$$fywd = 250 + 185.\frac{(h-15)}{20} = 250 + 185 \times \frac{(16cm-15)}{20} = 259,20MPa \div 26\frac{KN}{cm^2}$$

$$Trd_{3} = 0.10.\left(1 + \sqrt{\frac{20}{d}}\right).(100.\rho.fck)^{\frac{1}{3}} + 1.5.\frac{d}{s_{r}}.\frac{Asw.fywd.sen}{u.d}$$

=
$$0.10 \times \left(1 + \sqrt{\frac{20}{11.5cm}}\right) \times (100 \times 0.0062 \times 25MPa)^{\frac{1}{3}} = 0.58MPa$$

$$=1.5\times\frac{11.5cm}{8cm}\times\frac{Asw\times259.2MPa\times1}{156.76cm\times11.5cm}=0.31\times Asw\times\frac{MPa}{cm^2}$$

$$Trd_3 = 0.58MPa + 0.31 \times Asw \times \frac{MPa}{cm^2}$$

 $Tsd \leq Trd_3$

$$1,32MPa \le 0,58MPa + 0,31 \times Asw \times \frac{MPa}{cm^2}$$

 $Asw \ge 2.39cm^2$

2.2 – Cálculo da armadura de punção obrigatória:

$$Asw \geq \frac{1}{2}.\frac{Fsd}{fywd} :: Asw \geq \frac{1}{2} \times \frac{238,50KN}{26\frac{KN}{cm^2}} :: Asw \geq 4,59cm^2$$

Serão no mínimo 3 contornos de armação à punção, então:

$$Asw \ge \frac{4,59cm^2}{3\ contornos} = 1,53cm^2$$

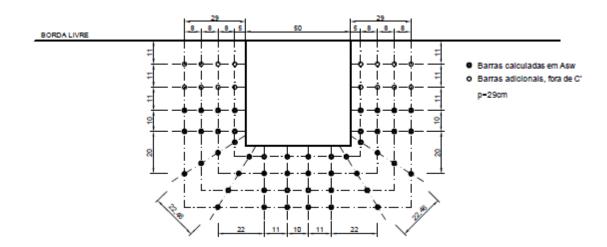
 $Asw : 2,39cm^2 > 1,53cm^2 : ok!$

2.3 – Disposição construtiva:

$$s_0 \leq 0.5.d :: s_0 \leq 0.5 \times 11.5cm :: s_0 \leq 5.75cm :: s_0 = 5.00cm$$

$$s_r \leq 0.75.d : s_r \leq 0.75 \times 11.5cm : s_r \leq 8.63cm : s_r = 8.00cm$$

$$s_e \leq 2.\,d :: s_e \leq 2 \times 11,5 cm :: s_e \leq 23 cm :: s_e = 23,00 cm$$



 $1205.0mm = 2,40cm^2 > 2,39cm^2 : ok! : Por contorno serão 1605.0mm$

3 – Verificação do Contorno C":

$$p = 29cm$$

$$u^* = 2.a + C_2 + 2.\pi.d + \pi.p$$

$$u^* = 2 \times 17,25cm + 50cm + 2 \times \pi \times 11,5cm + \pi \times 29cm$$

$$u^* = 247,86cm$$

$$\begin{split} wp_1 &= \frac{{C_1}^2}{2} + \frac{{C_1} \cdot {C_2}}{2} + 2.\,{C_2} \cdot d + 8.\,d^2 + \pi.\,d.\,{C_1} + {C_2} \cdot p + 8.\,d.\,p + \frac{\pi.\,p.\,{C_1}}{2} + 2.\,p^2 \\ &= \frac{(50cm)^2}{2} + \frac{50cm \times 50cm}{2} + 2 \times 50cm \times 11,5cm + 8 \times (11,5cm)^2 = 4708cm^2 \end{split}$$

$$= \pi \times 11,5cm \times 50cm + 50cm \times 29cm + 8 \times 11,5cm \times 29cm = 5924,42cm^2$$

$$= \frac{\pi \times 29cm \times 50cm}{2} + 2 \times (29cm)^2 = 3959,65cm^2$$

$$wp_1 = 4708cm^2 + 5924,42cm^2 + 3959,65cm^2 = 14592,07cm^2$$

$$wp_2 = \frac{{C_2}^2}{4} + C_1.\,C_2 + 4.\,C_1.\,\mathrm{d} + 8.\,\mathrm{d}^2 + \pi.\,\mathrm{d}.\,C_2 + 2.\,C_1.\,p + 8.\,d.\,p + \frac{\pi.\,p.\,C_2}{2} + 2.\,p^2$$

$$= \frac{(50cm)^2}{4} + 50cm \times 50cm + 4 \times 50cm \times 11,5cm + 8 \times (11,5cm)^2 = 6483cm^2$$

$$=\pi \times 11,5cm \times 50cm + 2 \times 50cm \times 29cm + 8 \times 11,5cm \times 29cm = 7374,42cm^2$$

$$= \frac{\pi \times 29cm \times 50cm}{2} + 2 \times (29cm)^2 = 3959,65cm^2$$

$$wp_2 = 6483cm^2 + 7374,42cm^2 + 3959,65cm^2 = 17817,07cm^2$$

$$e^* = \frac{C_1.a - a^2 + \frac{C_1 + C_2}{2} + 2.C_2.d + 8.d^2 + \pi.d.C_1 + C_2.p + 8.d.p + \frac{\pi.p.C_1}{2} + 2.p^2}{2.a + C_2 + 2.\pi.d + \pi.p}$$

$$=50cm \times 17,25cm - (17,25cm)^2 + \frac{50cm + 50cm}{2} + 2 \times 50cm \times 11,5cm = 1764,94cm^2$$

$$= 8 \times (11,5cm)^2 + \pi \times 11,5cm \times 50cm + 50cm \times 29cm = 4314,42cm^2$$

$$= 8 \times 11,5 cm \times 29 cm + \frac{\pi \times 29 cm \times 50 cm}{2} + 2 \times (29 cm)^{2} = 6627,65 cm^{2}$$

$$= 2 \times 17,25cm + 50cm + 2 \times \pi \times 11,5cm + \pi \times 29cm = 247,86cm$$

$$e^* = \frac{1764,94cm^2 + 4314,42cm^2 + 6627,65cm^2}{247,86cm} = 51,27m$$

$$Msd_1^* = Fsd.e^* = 238,5KN \times 51,27cm = 12227,90KN.cm$$

$$Msd_{1fim} \geq \begin{cases} 0 \\ Msd_{1} - Msd_{1}^{*} = 5335,4KN.cm - 12227,9KN.cm = -6892,5KN.cm \end{cases}$$

$$Msd_{1fim} = 0$$

$$\frac{C_1}{C_2} = \frac{50cm}{50cm} = 1 : K_1 = 0.60$$

$$\frac{C_2}{2.C_4} = \frac{50cm}{2 \times 50cm} = 0.5 : K_2 = 0.45$$

$$Tsd = \frac{Fsd}{u^*.d} + K_1.\frac{Msd_{1_{fim}}}{wp_1.d} + K_2.\frac{Msd_2}{wp_2.d}$$

$$Tsd = \frac{238,50KN}{247,86cm \times 11,5cm} + 0,6.\frac{0}{14592,07cm^2 \times 11,5cm} + 0,45.\frac{0}{17817,07cm^2 \times 11,5cm}$$

$$Tsd = 0.084 \frac{KN}{cm^2} = 0.84MPa$$

$$Trd_1 = 0.75MPa$$

 $Tsd = 0.84MPa > Trd_1 = 0.75MPa : Colocar mais um contorno de armação!$

4 – Inserir mais um contorno de armação:

Obs: Como o momento final é nulo e continuará sendo nulo com o aumento da excentricidade, consegue-se estimar o valor de "p" para que a solicitação seja menor que a resistência.

$$Tsd \leq Trd_1 : Tsd \leq 0.75MPa : Tsd \leq 0.075 \frac{KN}{cm^2}$$

$$Tsd = \frac{Fsd}{u^*.d} + 0 = \frac{238,5KN}{u^*.11.5cm} = \frac{20,74}{u^*} \frac{KN}{cm}$$

$$\frac{20,74}{u^*} \frac{KN}{cm} \le 0,075 \frac{KN}{cm^2} :: u^* \ge 276,53cm$$

$$u^* = 2.a + C_2 + 2.\pi.d + \pi.p$$

$$u^* = 2 \times 17,25cm + 50cm + 2 \times \pi \times 11,5cm + \pi \times p$$

$$u^* = 156,76cm + \pi \times p$$

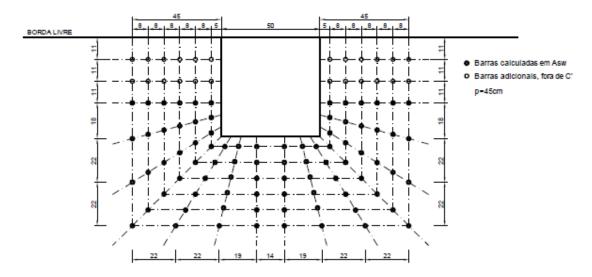
$$156,76cm + \pi \times p \ge 276,53cm : p \ge 38,12cm$$

5 – Nova disposição construtiva:

$$s_0 \leq 0.5.\,d \, \div s_0 \leq 0.5 \times 11.5cm \, \div s_0 \leq 5.75cm \, \div s_0 = 5.00cm$$

$$s_r \le 0.75.d : s_r \le 0.75 \times 11.5cm : s_r \le 8.63cm : s_r = 8.00cm$$

$$s_e \leq 2.\,d :: s_e \leq 2 \times 11,\!5cm :: s_e \leq 23cm :: s_e = 23,\!00cm$$



 $1405.0mm = 2,74cm^2 > 2,39cm^2 : ok! : Por contorno serão 1805.0mm$

9 - Verificação do novo contorno C":

$$p = 45cm$$

$$u^* = 2.a + C_2 + 2.\pi.d + \pi.p$$

$$u^* = 2 \times 17,25cm + 50cm + 2 \times \pi \times 11,5cm + \pi \times 45cm$$

$$u^* = 298,13cm$$

$$\begin{split} wp_1 &= \frac{{C_1}^2}{2} + \frac{{C_1} \cdot {C_2}}{2} + 2.\,{C_2} \cdot d + 8.\,d^2 + \pi.\,d.\,{C_1} + {C_2} \cdot p + 8.\,d.\,p + \frac{\pi.\,p.\,{C_1}}{2} + 2.\,p^2 \\ &= \frac{(50cm)^2}{2} + \frac{50cm \times 50cm}{2} + 2 \times 50cm \times 11,5cm + 8 \times (11,5cm)^2 = 4708cm^2 \end{split}$$

$$=\pi\times11,5cm\times50cm+50cm\times45cm+8\times11,5cm\times45cm=8196,42cm^2$$

$$= \frac{\pi \times 45cm \times 50cm}{2} + 2 \times (45cm)^2 = 7584,29cm^2$$

$$wp_1 = 4708cm^2 + 8196,42cm^2 + 7584,29cm^2 = 20488,71cm^2$$

$$wp_2 = \frac{{C_2}^2}{4} + C_1.\,C_2 + 4.\,C_1.\,d + 8.\,d^2 + \pi.\,d.\,C_2 + 2.\,C_1.\,p + 8.\,d.\,p + \frac{\pi.\,p.\,C_2}{2} + 2.\,p^2$$

$$= \frac{(50cm)^2}{4} + 50cm \times 50cm + 4 \times 50cm \times 11,5cm + 8 \times (11,5cm)^2 = 6483cm^2$$

$$= \pi \times 11,5cm \times 50cm + 2 \times 50cm \times 45cm + 8 \times 11,5cm \times 45cm = 10446,42cm^2$$

$$= \frac{\pi \times 45cm \times 50cm}{2} + 2 \times (45cm)^2 = 7584,29cm^2$$

$$wp_2 = 6483cm^2 + 10446,\!42cm^2 + 7584,\!29cm^2 = 24513,\!71cm^2$$

$$e^* = \frac{C_1.a - a^2 + \frac{C_1 + C_2}{2} + 2.C_2.d + 8.d^2 + \pi.d.C_1 + C_2.p + 8.d.p + \frac{\pi.p.C_1}{2} + 2.p^2}{2.a + C_2 + 2.\pi.d + \pi.p}$$

$$=50cm \times 17,25cm - (17,25cm)^2 + \frac{50cm + 50cm}{2} + 2 \times 50cm \times 11,5cm = 1764,94cm^2$$

$$= 8 \times (11,5cm)^2 + \pi \times 11,5cm \times 50cm + 50cm \times 45cm = 5114,42cm^2$$

$$= 8 \times 11,5 cm \times 45 cm + \frac{\pi \times 45 cm \times 50 cm}{2} + 2 \times (45 cm)^{2} = 11724,29 cm^{2}$$

$$= 2 \times 17,25cm + 50cm + 2 \times \pi \times 11,5cm + \pi \times 45cm = 298,13cm$$

$$e^* = \frac{1764,94cm^2 + 5114,42cm^2 + 11724,29cm^2}{298,13cm} = 62,4cm$$

$$Msd_1^* = Fsd.e^* = 238,5KN \times 62,4cm = 14882,4KN.cm$$

$$Msd_{1fim} \geq \begin{cases} 0 \\ Msd_{1} - Msd_{1}^{*} = 5335,4KN.cm - 14882,4KN.cm = -9547KN.cm \end{cases}$$

$$Msd_{1fim} = 0$$

$$\frac{C_1}{C_2} = \frac{50cm}{50cm} = 1 :: K_1 = 0,60$$

$$\frac{C_2}{2.\,C_1} = \frac{50\,cm}{2\times50\,cm} = 0.5 \, \therefore \, K_2 = 0.45$$

$$Tsd = \frac{Fsd}{u^*.d} + K_1.\frac{Msd_{1_{fim}}}{wp_1.d} + K_2.\frac{Msd_2}{wp_2.d}$$

$$Tsd = \frac{238,50KN}{298,13cm \times 11,5cm} + 0,6.\frac{0}{20488,71cm^2 \times 11,5cm} + 0,45.\frac{0}{24513,71cm^2 \times 11,5cm}$$

$$Tsd = 0.070 \frac{KN}{cm^2} = 0.70 MPa$$

$$Trd_1 = 0.75MPa$$

$$Tsd = 0.70MPa < Trd_1 = 0.75MPa : ok!$$

Verificação dos pilares P4 e P6, usando como referência o pilar P4:

1 - Dados:

$$C_1 = 50cm$$

$$C_2 = 50cm$$

$$d = h - d' = 16cm - 4.5cm = 11.5cm$$

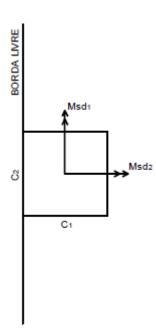
$$Fsd = Fz. 1,4 = 178,84KN \times 1,4 = 250,38KN (Compressão)$$

$$Msd_1 = M_v. 1,4 = 2706KN.cm \times 1,4 = 3788,4KN.cm$$

$$Msd_2 = M_x$$
. 1,4 = 0 KN. cm × 1,4 = 0 KN. cm

$$a_0 \leq 1{,}5.\,d$$
 .: $a_0 \leq 1{,}5 \times 11{,}5cm$.: $a_0 \leq 17{,}25cm$.: $a_0 = 17{,}25cm$

$$\alpha \leq \begin{cases} 1, 5.d = 1, 5 \times 11, 5cm = 17, 25cm \\ 0, 5. \, C_2 = 0, 5 \times 50cm = 25cm \end{cases} \ \therefore \ \alpha = 17, 25cm$$



2 – Verificação do Contorno C:

$$u^* = 2.a_0 + C_2 = 2 \times 17,25cm + 50cm = 84,5cm$$

$$wp_1 = \frac{{C_1}^2}{2} + \frac{{C_1} \cdot {C_2}}{2} = \frac{(50cm)^2}{2} + \frac{50cm \times 50cm}{2} = 2500cm^2$$

$$wp_2 = \frac{{C_2}^2}{4} + C_1$$
. $C_2 = \frac{(50cm)^2}{4} + 50cm \times 50cm = 3125cm^2$

$$e^* = \frac{{C_1}.{a_0} - {a_0}^2 + \frac{{C_1}.{C_2}}{2}}{2.\,a_0 + {C_2}} = \frac{50cm \times 17,25cm - (17,25cm)^2 + \frac{50cm \times 50cm}{2}}{2 \times 17,25cm + 50cm}$$

$$e^* = 21,48cm$$

$$Msd_1^* = Fsd.e^* = 250,38KN \times 21,48cm = 5378,16KN.cm$$

$$Msd_{1fim} \geq \begin{cases} 0 \\ Msd_{1} - Msd_{1}^{*} = 3788,4KN.cm - 5378,16KN.cm = -1589,76KN.cm \end{cases}$$

$$Msd_{1fim} = 0$$

$$\frac{C_1}{C_2} = \frac{50cm}{50cm} = 1 : K_1 = 0.60$$

$$\frac{C_2}{2.\,C_1} = \frac{50\,cm}{2\times50\,cm} = 0.5 \, \therefore K_2 = 0.45$$

$$Tsd = \frac{Fsd}{u^*.d} + K_1.\frac{Msd_{1_{fim}}}{wp_1.d} + K_2.\frac{Msd_2}{wp_2.d}$$

$$Tsd = \frac{250,38KN}{84,5cm \times 11,5cm} + 0.6 \times \frac{0}{2500cm^2 \times 11,5cm} + 0.45 \times \frac{0}{3125cm^2 \times 11,5cm}$$

$$Tsd = 0.258 \frac{KN}{cm^2} = 2.58MPa$$

$$Trd_2 = 0.27. \propto_v. fcd = 0.27 \times \left(1 - \frac{fck}{250}\right) \times \left(\frac{fck}{1.4}\right) = 0.27 \times \left(1 - \frac{25MPa}{250}\right) \times \left(\frac{25MPa}{1.4}\right)$$

$$Trd_2 = 4,34MPa$$

$$Tsd = 2,58MPa < Trd_2 = 4,34MPa : ok!$$

3 – Verificação do Contorno C':

$$u^* = 2.a + C_2 + 2.\pi.d = 2 \times 17,25cm + 50cm + 2 \times \pi \times 11,5cm = 156,76cm$$

$$wp_1 = \frac{{C_1}^2}{2} + \frac{{C_1} \cdot {C_2}}{2} + 2 \cdot {C_2} \cdot d + 8 \cdot d^2 + \pi \cdot d \cdot C_1$$

$$= \frac{(50cm)^2}{2} + \frac{50cm \times 50cm}{2} + 2 \times 50cm \times 11,5cm = 3650cm^2$$

$$= 8 \times (11,5cm)^2 + \pi \times 11,5cm \times 50cm = 2864,42cm^2$$

$$wp_1 = 3650cm^2 + 2864,42cm^2 = 6514,42cm^2$$

$$wp_2 = \frac{{C_2}^2}{4} + C_1.C_2 + 4.C_1.d + 8.d^2 + \pi.d.C_2$$

$$= \frac{(50cm)^2}{4} + 50cm \times 50cm + 4 \times 50cm \times 11,5cm = 5425cm^2$$

$$= 8 \times (11,5cm)^2 + \pi \times 11,5cm \times 50cm = 2864,42cm^2$$

$$wp_2 = 5425cm^2 + 2864,42cm^2 = 8289,42cm^2$$

$$e^* = \frac{C_1.a - a^2 + \frac{C_1 + C_2}{2} + 2.C_2.d + 8.d^2 + \pi.d.C_1}{2.a + C_2 + 2.\pi.d} =$$

$$= 50cm \times 17,25cm - (17,25cm)^2 + \frac{50cm + 50cm}{2} = 614,94cm^2$$

$$= 2 \times 50cm \times 11,5cm + 8 \times (11,5cm)^2 + \pi \times 11,5cm \times 50cm = 4014,42cm^2$$

$$= 2 \times 17,25cm + 50cm + 2 \times \pi \times 11,5cm = 156,76cm$$

$$e^* = \frac{614,94cm^2 + 4014,42cm^2}{156,76cm} = 29,53cm$$

$$Msd_1^* = Fsd.e^* = 250,38KN \times 29,53cm = 7393,72KN.cm$$

$$Msd_{1fim} \geq \begin{cases} 0 \\ Msd_{1} - Msd_{1}^{*} = 3788,4KN.cm - 7393,72KN.cm = -3605,32KN.cm \end{cases}$$

$$Msd_{1_{fim}} = 0$$

$$\frac{C_1}{C_2} = \frac{50cm}{50cm} = 1 :: K_1 = 0.60$$

$$\frac{C_2}{2.C_1} = \frac{50cm}{2 \times 50cm} = 0.5 : K_2 = 0.45$$

$$Tsd = \frac{Fsd}{u^*.d} + K_1.\frac{Msd_{1_{fim}}}{wp_1.d} + K_2.\frac{Msd_2}{wp_2.d}$$

$$Tsd = \frac{250,38KN}{156,76cm \times 11,5cm} + 0.6 \times \frac{0}{6514,42cm^2 \times 11,5cm} + 0.45 \times \frac{0}{8289,42cm^2 \times 11,5cm}$$

$$Tsd = 0.139 \frac{KN}{cm^2} = 1.39MPa$$

$$\emptyset 10mm ~\grave{a}~cada~14cm ~ : ~ \rho_x = \frac{As_x}{Ac} = \frac{0.8cm^2}{14cm \times 16cm} = 0.0036$$

Ø16mm à cada 13cm :
$$\rho_y = \frac{As_y}{Ac} = \frac{2cm^2}{13cm \times 16cm} = 0,0096$$

$$\rho = \sqrt{\rho_x \cdot \rho_y} = \sqrt{0.0036 \times 0.0096} = 0.0059$$

$$Trd_1 = 0.13.\left(1 + \sqrt{\frac{20}{d}}\right).\left(100.\rho.fck\right)^{\frac{1}{3}}$$

$$Trd_1 = 0.13 \times \left(1 + \sqrt{\frac{20}{11.5cm}}\right) \times (100 \times 0.0062 \times 25MPa)^{\frac{1}{3}} = 0.75MPa$$

$$Tsd = 1,39MPa > Trd_1 = 0,75MPa : Armar à punção!$$

3.1 – Cálculo da armadura de punção:

$$\alpha = 90^{\circ}$$
 : $sen 90^{\circ} = 1.0$

$$s_r \le 0.75.d : s_r \le 0.75 \times 11.5cm : s_r \le 8.63cm : s_r = 8.0cm$$

$$fywd = 250 + 185.\frac{(h-15)}{20} = 250 + 185 \times \frac{(16cm-15)}{20} = 259,20MPa \div 26\frac{KN}{cm^2}$$

$$Trd_{3} = 0.10.\left(1 + \sqrt{\frac{20}{d}}\right).\left(100.\rho.fck\right)^{\frac{1}{3}} + 1.5.\frac{d}{s_{r}}.\frac{Asw.fywd.sen}{u.d} \propto \frac{1}{s_{r}}$$

$$= 0.10 \times \left(1 + \sqrt{\frac{20}{11.5cm}}\right) \times (100 \times 0.0059 \times 25MPa)^{\frac{1}{3}} = 0.57MPa$$

$$= 1.5 \times \frac{11.5cm}{8cm} \times \frac{Asw \times 259.2MPa \times 1}{156.76cm \times 11.5cm} = 0.31 \times Asw \times \frac{MPa}{cm^2}$$

$$Trd_3 = 0.57MPa + 0.31 \times Asw \times \frac{MPa}{cm^2}$$

 $Tsd \leq Trd_3$

$$1,4MPa \le 0,57MPa + 0,31 \times Asw \times \frac{MPa}{cm^2}$$

$$Asw \ge 2.68cm^2$$

3.2 – Cálculo da armadura de punção obrigatória:

$$Asw \ge \frac{1}{2} \cdot \frac{Fsd}{fywd} :: Asw \ge \frac{1}{2} \times \frac{250,38KN}{26\frac{KN}{cm^2}} :: Asw \ge 4,82cm^2$$

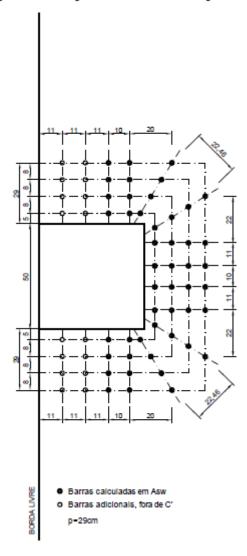
Serão no mínimo 3 contornos de armação à punção, então:

$$Asw \ge \frac{4,82cm^2}{3\ contornos} = 1,61cm^2$$

$$Asw :: 2,68cm^2 > 1,61cm^2 :: ok!$$

3.3 – Disposição construtiva:

$$\begin{split} s_0 &\leq 0.5.\,d \, \div s_0 \leq 0.5 \times 11.5cm \, \div s_0 \leq 5.75cm \, \div s_0 = 5.00cm \\ s_r &\leq 0.75.\,d \, \div s_r \leq 0.75 \times 11.5cm \, \div s_r \leq 8.63cm \, \div s_r = 8.00cm \\ s_e &\leq 2.\,d \, \div s_e \leq 2 \times 11.5cm \, \div s_e \leq 23cm \, \div s_e = 23.00cm \end{split}$$



 $12\emptyset6.3mm = 3,78cm^2 > 2,68cm^2 : ok! : Por contorno serão 16\Ø6.3mm$

4 – Verificação do Contorno C":

$$p = 29cm$$

$$u^* = 2.a + C_2 + 2.\pi.d + \pi.p$$

$$u^* = 2 \times 17,25cm + 50cm + 2 \times \pi \times 11,5cm + \pi \times 29cm$$

$$u^* = 247,86cm$$

$$wp_1 = \frac{{C_1}^2}{2} + \frac{{C_1} \cdot {C_2}}{2} + 2 \cdot {C_2} \cdot d + 8 \cdot d^2 + \pi \cdot d \cdot C_1 + C_2 \cdot p + 8 \cdot d \cdot p + \frac{\pi \cdot p \cdot C_1}{2} + 2 \cdot p^2$$

$$= \frac{(50cm)^2}{2} + \frac{50cm \times 50cm}{2} + 2 \times 50cm \times 11,5cm + 8 \times (11,5cm)^2 = 4708cm^2$$

$$= \pi \times 11,5cm \times 50cm + 50cm \times 29cm + 8 \times 11,5cm \times 29cm = 5924,42cm^2$$

$$= \frac{\pi \times 29cm \times 50cm}{2} + 2 \times (29cm)^2 = 3959,65cm^2$$

$$wp_1 = 4708cm^2 + 5924,42cm^2 + 3959,65cm^2 = 14592,07cm^2$$

$$wp_2 = \frac{{C_2}^2}{4} + C_1.\,C_2 + 4.\,C_1.\,d + 8.\,d^2 + \pi.\,d.\,C_2 + 2.\,C_1.\,p + 8.\,d.\,p + \frac{\pi.\,p.\,C_2}{2} + 2.\,p^2$$

$$= \frac{(50cm)^2}{4} + 50cm \times 50cm + 4 \times 50cm \times 11,5cm + 8 \times (11,5cm)^2 = 6483cm^2$$

$$= \pi \times 11,5cm \times 50cm + 2 \times 50cm \times 29cm + 8 \times 11,5cm \times 29cm = 7374,42cm^2$$

$$= \frac{\pi \times 29cm \times 50cm}{2} + 2 \times (29cm)^2 = 3959,65cm^2$$

$$wp_2 = 6483cm^2 + 7374,\!42cm^2 + 3959,\!65cm^2 = 17817,\!07cm^2$$

$$e^* = \frac{C_1.a - a^2 + \frac{C_1 + C_2}{2} + 2.C_2.d + 8.d^2 + \pi.d.C_1 + C_2.p + 8.d.p + \frac{\pi.p.C_1}{2} + 2.p^2}{2.a + C_2 + 2.\pi.d + \pi.p}$$

$$=50cm \times 17,25cm - (17,25cm)^2 + \frac{50cm + 50cm}{2} + 2 \times 50cm \times 11,5cm = 1764,94cm^2$$

$$= 8 \times (11,5cm)^2 + \pi \times 11,5cm \times 50cm + 50cm \times 29cm = 4314,42cm^2$$

$$= 8 \times 11,5 cm \times 29 cm + \frac{\pi \times 29 cm \times 50 cm}{2} + 2 \times (29 cm)^{2} = 6627,65 cm^{2}$$

$$= 2 \times 17,25cm + 50cm + 2 \times \pi \times 11,5cm + \pi \times 29cm = 247,86cm$$

$$e^* = \frac{1764,94cm^2 + 4314,42cm^2 + 6627,65cm^2}{247.86cm} = 51,27m$$

$$Msd_1^* = Fsd.e^* = 250,38KN \times 51,27cm = 12836,98KN.cm$$

$$Msd_{1fim} \geq \begin{cases} 0 \\ Msd_{1} - Msd_{1}^{*} = 3788,4KN.cm - 12836,98KN.cm = -9048,58KN.cm \end{cases}$$

$$Msd_{1_{fim}} = 0$$

$$\frac{C_1}{C_2} = \frac{50\,cm}{50\,cm} = 1 \, \therefore \, K_1 = 0,60$$

$$\frac{C_2}{2.\,C_1} = \frac{50\,cm}{2\times50\,cm} = 0.5 \, \therefore K_2 = 0.45$$

$$Tsd = \frac{Fsd}{u^*.d} + K_1.\frac{Msd_{1_{fim}}}{wp_1.d} + K_2.\frac{Msd_2}{wp_2.d}$$

$$Tsd = \frac{250,38KN}{247,86cm \times 11,5cm} + 0,6.\frac{0}{14592,07cm^2 \times 11,5cm} + 0,45.\frac{0}{17817,07cm^2 \times 11,5cm}$$

$$Tsd = 0.088 \frac{KN}{cm^2} = 0.88MPa$$

$$Trd_1 = 0.75MPa$$

 $Tsd = 0.88MPa > Trd_1 = 0.75MPa : Colocar mais um contorno de armação!$

5 – Inserir mais um contorno de armação:

Obs: Como o momento final é nulo e continuará sendo nulo com o aumento da excentricidade, consegue-se estimar o valor de "p" para que a solicitação seja menor que a resistência.

$$Tsd \leq Trd_1 : Tsd \leq 0.75MPa : Tsd \leq 0.075 \frac{KN}{cm^2}$$

$$Tsd = \frac{Fsd}{u^*.d} + 0 = \frac{250,38KN}{u^*.11,5cm} = \frac{21,77}{u^*} \frac{KN}{cm}$$

$$\frac{21,77}{u^*} \frac{KN}{cm} \leq 0,075 \frac{KN}{cm^2} :: u^* \geq 290,27cm$$

$$u^* = 2.a + C_2 + 2.\pi.d + \pi.p$$

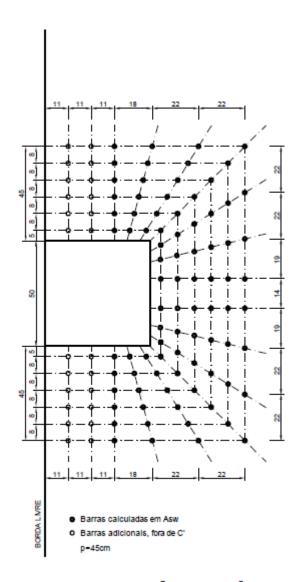
$$u^* = 2 \times 17,25cm + 50cm + 2 \times \pi \times 11,5cm + \pi \times p$$

$$u^* = 156,76cm + \pi \times p$$

 $156,76cm + \pi \times p \ge 290,27cm : p \ge 42,51cm$

6 – Nova disposição construtiva:

$$\begin{split} s_0 &\leq 0,5.\,d \, :: s_0 \leq 0,5 \times 11,5 cm \, :: s_0 \leq 5,75 cm \, :: s_0 = 5,00 cm \\ \\ s_r &\leq 0,75.\,d \, :: s_r \leq 0,75 \times 11,5 cm \, :: s_r \leq 8,63 cm \, :: s_r = 8,00 cm \\ \\ s_e &\leq 2.\,d \, :: s_e \leq 2 \times 11,5 cm \, :: s_e \leq 23 cm \, :: s_e = 23,00 cm \end{split}$$



 $1405.0mm = 2,74cm^2 > 2,68cm^2 : ok! : Por contorno serão 1805.0mm$

7 – Verificação do novo contorno C":

$$p = 45cm$$

$$u^* = 2.a + C_2 + 2.\pi.d + \pi.p$$

$$u^* = 2 \times 17,25cm + 50cm + 2 \times \pi \times 11,5cm + \pi \times 45cm$$

$$u^* = 298,13cm$$

$$wp_1 = \frac{{C_1}^2}{2} + \frac{{C_1} \cdot {C_2}}{2} + 2 \cdot {C_2} \cdot d + 8 \cdot d^2 + \pi \cdot d \cdot C_1 + C_2 \cdot p + 8 \cdot d \cdot p + \frac{\pi \cdot p \cdot C_1}{2} + 2 \cdot p^2$$
$$= \frac{(50cm)^2}{2} + \frac{50cm \times 50cm}{2} + 2 \times 50cm \times 11,5cm + 8 \times (11,5cm)^2 = 4708cm^2$$

$$= \pi \times 11,5cm \times 50cm + 50cm \times 45cm + 8 \times 11,5cm \times 45cm = 8196,42cm^{2}$$

$$= \frac{\pi \times 45cm \times 50cm}{2} + 2 \times (45cm)^2 = 7584,29cm^2$$

$$wp_1 = 4708cm^2 + 8196,42cm^2 + 7584,29cm^2 = 20488,71cm^2$$

$$wp_2 = \frac{{C_2}^2}{4} + C_1.\,C_2 + 4.\,C_1.\,d + 8.\,d^2 + \pi.\,d.\,C_2 + 2.\,C_1.\,p + 8.\,d.\,p + \frac{\pi.\,p.\,C_2}{2} + 2.\,p^2$$

$$= \frac{(50cm)^2}{4} + 50cm \times 50cm + 4 \times 50cm \times 11,5cm + 8 \times (11,5cm)^2 = 6483cm^2$$

$$= \pi \times 11,5cm \times 50cm + 2 \times 50cm \times 45cm + 8 \times 11,5cm \times 45cm = 10446,42cm^{2}$$

$$= \frac{\pi \times 45cm \times 50cm}{2} + 2 \times (45cm)^2 = 7584,29cm^2$$

$$wp_2 = 6483cm^2 + 10446,42cm^2 + 7584,29cm^2 = 24513,71cm^2$$

$$e^* = \frac{C_1.a - a^2 + \frac{C_1 + C_2}{2} + 2.C_2.d + 8.d^2 + \pi.d.C_1 + C_2.p + 8.d.p + \frac{\pi.p.C_1}{2} + 2.p^2}{2.a + C_2 + 2.\pi.d + \pi.p}$$

$$=50cm \times 17,25cm - (17,25cm)^2 + \frac{50cm + 50cm}{2} + 2 \times 50cm \times 11,5cm = 1764,94cm^2$$

$$= 8 \times (11,5cm)^2 + \pi \times 11,5cm \times 50cm + 50cm \times 45cm = 5114,42cm^2$$

$$= 8 \times 11,5cm \times 45cm + \frac{\pi \times 45cm \times 50cm}{2} + 2 \times (45cm)^2 = 11724,29cm^2$$

$$= 2 \times 17,25cm + 50cm + 2 \times \pi \times 11,5cm + \pi \times 45cm = 298,13cm$$

$$e^* = \frac{1764,94cm^2 + 5114,42cm^2 + 11724,29cm^2}{298,13cm} = 62,4cm$$

$$Msd_1^* = Fsd.e^* = 250,38KN \times 62,4cm = 15623,71KN.cm$$

$$Msd_{1fim} \geq \begin{cases} 0 \\ Msd_{1} - Msd_{1}^{*} = 3788,4KN.cm - 15623,71KN.cm = -11835,31KN.cm \end{cases}$$

$$Msd_{1fim} = 0$$

$$\frac{C_1}{C_2} = \frac{50cm}{50cm} = 1 : K_1 = 0,60$$

$$\frac{C_2}{2.\,C_1} = \frac{50\,cm}{2\times50\,cm} = 0.5 \, \therefore K_2 = 0.45$$

$$Tsd = \frac{Fsd}{u^*.d} + K_1.\frac{Msd_{1_{fim}}}{wp_1.d} + K_2.\frac{Msd_2}{wp_2.d}$$

$$Tsd = \frac{250,38KN}{298,13cm \times 11,5cm} + 0,6.\frac{0}{20488,71cm^2 \times 11,5cm} + 0,45.\frac{0}{24513,71cm^2 \times 11,5cm}$$

$$Tsd = 0.073 \frac{KN}{cm^2} = 0.73MPa$$

$$Trd_1 = 0.75MPa$$

$$Tsd = 0.73MPa < Trd_1 = 0.75MPa : ok!$$

Verificação do pilar P5:



$$C_1 = 50cm$$

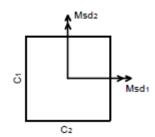
$$C_2 = 50cm$$

$$d = h - d' = 16cm - 4,5cm = 11,5cm$$

$$Fsd = Fz. 1,4 = 253,38KN \times 1,4 = 354,73KN (Compressão)$$

$$Msd_1 = M_x.1,4 = 0 \ KN. \ cm \times 1,4 = 0 \ KN. \ cm$$

$$Msd_2 = M_v. 1,4 = 0 \ KN. \ cm \times 1,4 = 0 \ KN. \ cm$$



2 – Verificação do Contorno C:

$$u = 2.(C_1 + C_2) = 2 \times (50cm + 50cm) = 200cm$$

$$wp_1 = \frac{{C_1}^2}{2} + C_1 \cdot C_2 = \frac{(50cm)^2}{2} + 50cm \times 50cm = 3750cm^2$$

$$wp_2 = \frac{{C_2}^2}{2} + C_2$$
. $C_1 = \frac{(50cm)^2}{2} + 50cm \times 50cm = 3750m^2$

$$\frac{C_1}{C_2} = \frac{50\,cm}{50\,cm} = 1 \, \therefore \, K_1 = 0,60$$

$$\frac{C_2}{C_1} = \frac{50cm}{50cm} = 1 : K_2 = 0.60$$

$$Tsd = \frac{Fsd}{u.d} + K_1 \cdot \frac{Msd_1}{wp_1.d} + K_2 \cdot \frac{Msd_2}{wp_2.d}$$

$$Tsd = \frac{354,73KN}{200cm \times 11,5cm} + 0.6 \times \frac{0}{3750cm^2 \times 11,5cm} + 0.6 \times \frac{0}{3750cm^2 \times 11,5cm}$$

$$Tsd = 0.154 \frac{KN}{cm^2} = 1.54 MPa$$

$$Trd_2 = 0.27. \propto_v fcd = 0.27 \times \left(1 - \frac{fck}{250}\right) \times \left(\frac{fck}{1.4}\right) = 0.27 \times \left(1 - \frac{25MPa}{250}\right) \times \left(\frac{25MPa}{1.4}\right)$$

 $Trd_2 = 4,34MPa$

$$Tsd = 1.54MPa < Trd_2 = 4.34MPa : ok!$$

3 – Verificação do Contorno C':

$$u = 2.\left(C_{1} + C_{2}\right) + 4.\pi. d = 2 \times \left(50cm + 50cm\right) + 4 \times \pi \times 11,5cm = 344,51cm$$

$$wp_1 = \frac{{C_1}^2}{2} + C_1 \cdot C_2 + 4 \cdot C_2 \cdot d + 16 \cdot d^2 + 2 \cdot \pi \cdot d \cdot C_1$$

$$= \frac{(50cm)^2}{2} + 50cm \times 50cm + 4 \times 50cm \times 11,5cm = 6050cm^2$$

=
$$16 \times (11,5 \text{cm})^2 + 2 \times \pi \times 11,5 \text{cm} \times 50 \text{cm} = 5728,83 \text{cm}^2$$

$$wp_1 = 6050cm^2 + 5728,83cm^2 = 11778,83cm^2$$

$$wp_2 = \frac{{C_2}^2}{2} + C_2 \cdot C_1 + 4 \cdot C_1 \cdot d + 16 \cdot d^2 + 2 \cdot \pi \cdot d \cdot C_2$$
$$= \frac{(50cm)^2}{2} + 50cm \times 50cm + 4 \times 50cm \times 11,5cm = 6050cm^2$$

=
$$16 \times (11,5 \text{cm})^2 + 2 \times \pi \times 11,5 \text{cm} \times 50 \text{cm} = 5728,83 \text{cm}^2$$

$$wp_2 = 6050cm^2 + 5728,83cm^2 = 11778,83cm^2$$

$$\frac{C_1}{C_2} = \frac{50cm}{50cm} = 1 : K_1 = 0.60$$

$$\frac{C_2}{C_1} = \frac{50cm}{50cm} = 1 :: K_2 = 0.60$$

$$Tsd = \frac{Fsd}{u.d} + K_1 \cdot \frac{Msd_1}{wp_1 \cdot d} + K_2 \cdot \frac{Msd_2}{wp_2 \cdot d}$$

$$Tsd = \frac{354,73KN}{344,51cm \times 11,5cm} + 0.6 \times \frac{0}{11778,83m^2 \times 11,5cm} + 0.6 \times \frac{0}{11778,83cm^2 \times 11,5cm}$$

$$Tsd = 0.090 \frac{KN}{cm^2} = 0.90MPa$$

Ø12,5mm à cada 12cm :
$$\rho_x = \frac{As_x}{Ac} = \frac{1,25cm^2}{12cm \times 16cm} = 0,0065$$

$$\emptyset 16mm ~\grave{a}~cada~13cm ~ :. ~ \rho_y = \frac{As_y}{Ac} = \frac{2cm^2}{13cm \times 16cm} = 0,0096$$

$$\rho = \sqrt{\rho_x \cdot \rho_y} = \sqrt{0.0065 \times 0.0096} = 0.0079$$

$$Trd_1 = 0.13. \left(1 + \sqrt{\frac{20}{d}}\right). (100.\rho.fck)^{\frac{1}{3}}$$

$$Trd_1 = 0.13 \times \left(1 + \sqrt{\frac{20}{11.5cm}}\right) \times (100 \times 0.0079 \times 25MPa)^{\frac{1}{3}} = 0.81MPa$$

 $Tsd = 0.90MPa > Trd_1 = 0.81MPa : Armar à punção!$

3.1 – Cálculo da armadura de punção:

$$\alpha = 90^{\circ} : sen 90^{\circ} = 1.0$$

$$s_r \le 0.75.d : s_r \le 0.75 \times 11.5cm : s_r \le 8.63cm : s_r = 8.0cm$$

$$fywd = 250 + 185.\frac{(h-15)}{20} = 250 + 185 \times \frac{(16cm-15)}{20} = 259,20 MPa \div 26 \frac{KN}{cm^2}$$

$$Trd_3 = 0.10.\left(1 + \sqrt{\frac{20}{d}}\right).\left(100.\rho.fck\right)^{\frac{1}{3}} + 1.5.\frac{d}{s_r}.\frac{Asw.fywd.sen \propto u.d}{u.d}$$

$$= 0.10 \times \left(1 + \sqrt{\frac{20}{11.5cm}}\right) \times (100 \times 0.0079 \times 25MPa)^{\frac{1}{3}} = 0.63MPa$$

$$= 1.5 \times \frac{11.5cm}{8cm} \times \frac{Asw \times 259.2MPa \times 1}{344.51cm \times 11.5cm} = 0.14 \times Asw \times \frac{MPa}{cm^2}$$

$$Trd_3 = 0.63MPa + 0.14 \times Asw \times \frac{MPa}{cm^2}$$

 $Tsd \leq Trd_3$

$$0.90MPa \le 0.63MPa + 0.14 \times Asw \times \frac{MPa}{cm^2}$$

$$Asw > 1.93cm^2$$

3.2 – Cálculo da armadura de punção obrigatória:

$$Asw \ge \frac{1}{2} \cdot \frac{Fsd}{fywd} : Asw \ge \frac{1}{2} \times \frac{354,73KN}{26\frac{KN}{cm^2}} : Asw \ge 6,84cm^2$$

Serão no mínimo 3 contornos de armação à punção, então:

$$Asw \ge \frac{6,84cm^2}{3\ contornos} = 2,28cm^2$$

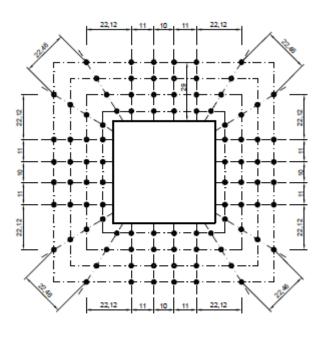
$$Asw : 1,93cm^2 < 2,28cm^2 : Não ok!$$

Obs: Portanto, se for detalhado apenas 3 contornos de armação, é necessário armar considerando 2,28cm² por contorno ao invés de 1,93cm² por contorno.

3.3 – Disposição construtiva:

$$s_0 \leq 0.5.\,d \, \div s_0 \leq 0.5 \times 11.5cm \, \div s_0 \leq 5.75cm \, \div s_0 = 5.00cm$$

$$\begin{split} s_r &\leq 0,75.d \ :: s_r \leq 0,75 \times 11,5cm \ :: s_r \leq 8,63cm \ :: s_r = 8,00cm \\ s_s &\leq 2.d \ :: s_s \leq 2 \times 11,5cm \ :: s_s \leq 23cm \ :: s_s = 23,00cm \end{split}$$



- Barras calculadas em Asw
- Barras adicionais, fora de C

 $2405.0mm = 4,80cm^2 > 1,93cm^2 : ok! : Por contorno serão 2405.0mm$

4 – Verificação do contorno C":

$$p = 29cm$$

$$u = 2.(C_1 + C_2) + 4.\pi.d + 2.\pi.p$$

$$u = 2 \times (50cm + 50cm) + 4 \times \pi \times 11,5cm + 2 \times \pi \times 29cm = 526,73cm$$

$$wp_1 = \frac{C_1^2}{2} + C_1 \cdot C_2 + 4 \cdot C_2 \cdot d + 16 \cdot d^2 + 2 \cdot \pi \cdot d \cdot C_1 + 2 \cdot C_2 \cdot p + 16 \cdot d \cdot p + 4 \cdot p^2 + \pi \cdot C_1 \cdot p$$

$$= \frac{(50cm)^2}{2} + 50cm \times 50cm + 4 \times 50cm \times 11,5cm + 16 \times (11,5cm)^2 = 8166cm^2$$

$$= 2 \times \pi \times 11,5cm \times 50cm + 2 \times 50cm \times 29cm = 6512,83cm^{2}$$

=
$$16 \times 11,5cm \times 29cm + 4 \times (29cm)^2 + \pi \times 50cm \times 29cm = 13255,31cm^2$$

$$wp_1 = 8166cm^2 + 6512,83cm^2 + 13255,31cm^2 = 27934,14cm^2$$

$$wp_2 = \frac{{C_2}^2}{2} + C_2 \cdot C_1 + 4 \cdot C_1 \cdot d + 16 \cdot d^2 + 2 \cdot \pi \cdot d \cdot C_2 + 2 \cdot C_1 \cdot p + 16 \cdot d \cdot p + 4 \cdot p^2 + \pi \cdot C_2 \cdot p$$

$$= \frac{(50cm)^2}{2} + 50cm \times 50cm + 4 \times 50cm \times 11,5cm + 16 \times (11,5cm)^2 = 8166cm^2$$

$$= 2 \times \pi \times 11,5cm \times 50cm + 2 \times 50cm \times 29cm = 6512,83cm^{2}$$

$$= 16 \times 11,5cm \times 29cm + 4 \times (29cm)^2 + \pi \times 50cm \times 29cm = 13255,31cm^2$$

$$wp_2 = 8166cm^2 + 6512,83cm^2 + 13255,31cm^2 = 27934,14cm^2$$

$$\frac{C_1}{C_2} = \frac{50cm}{50cm} = 1 : K_1 = 0.60$$

$$\frac{C_2}{C_1} = \frac{50cm}{50cm} = 1 :: K_2 = 0.60$$

$$Tsd = \frac{Fsd}{u.d} + K_1.\frac{Msd_1}{wp_1.d} + K_2.\frac{Msd_2}{wp_2.d}$$

$$Tsd = \frac{354,73KN}{526,73cm \times 11,5cm} + 0,6.\frac{0}{27934,14cm^2 \times 11,5cm} + 0,6.\frac{0}{27934,14cm^2 \times 11,5cm}$$

$$Tsd = 0.059 \frac{KN}{cm^2} = 0.59 MPa$$

$$Trd_1 = 0.81MPa$$

$$Tsd = 0.59MPa < Trd_1 = 0.81MPa : ok!$$